REPORT

Tonkin+Taylor



Geotechnical Assessment Report

Prepared for Christchurch City Council Prepared by Tonkin & Taylor Ltd Date November 2018 Job Number 1003207.v2





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Document Control

Title: Kyle	Park , Geo	technical Assessment Report			
Date	Version	Description	Prepared by:	Reviewed by:	Authorised by:
Nov 2018	1	Draft for CCC review	HJB	GGA	GGA
Nov 2018	2	Post CCC feedback	HJB	GGA	GGA

Distribution:

Christchurch City Council Tonkin & Taylor Ltd (FILE) 1 PDF copy 1 copy

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1 Introduction and Objectives

1.1 General

Tonkin & Taylor Ltd (T+T) was commissioned by the Christchurch City Council (CCC) to carry out a geotechnical assessment for the proposed combined Hornby Library, Customer Services and South West Leisure Facility (the Centre). This work has been completed in accordance with the CCC Statement of Work as agreed with T+T and dated 17 August 2018.

Kyle Park is located immediately adjacent to Waterloo Road in Hornby, which is in the south-west of Christchurch. The whole of Kyle Park was formerly a gravel pit that has been backfilled over a large number of years with a mix of uncontrolled fill materials (i.e. domestic, commercial and industrial waste materials).

CCC is considering developing the Centre on the eastern part of Kyle Park, with a preferred option being the eastern corner of the site, within the development area identified on Figure 1 (below), which identifies the following areas referred to in this report:

- Kyle Park red polygon below.
- The site (i.e. subject of this investigation and assessment) green polygon below.
- The *development area* (i.e. preferred location for the centre) blue polygon below.

The size The si

This report is generally focussed on the development area.

Figure 1 – Kyle Park location (source - <u>https://apps.canterburymaps.govt.nz/CanterburyHistoricAerialImagery/</u>).

1.2 Proposed development

Detailed design of the Centre has not been finalised at the time of writing this report. However, at a concept level the Centre is expected to include the following:

- Two storey service building and library, including community meeting rooms and offices.
- Leisure facility including swimming pools, courts and a sport hall.
- Car parking, landscaping areas and footpaths connecting the Centre to the remainder of Kyle Park and an underpass beneath the railway along the southern boundary with connection to the transport links at Hornby Hub.
- Provision of utilities/services to the development (e.g. water, power, telecommunications), which may include trenching across the current park.

The elevation of the existing ground level is approximately 36 m relative to the 1937 Lyttelton vertical datum (LVD). We understand that the ground floor level is to be set at approximately 38 mLVD to be consistent with the adjacent street level.

1.3 Objective and scope of work

The following scope of work has been completed by T+T for the purposes of this geotechnical assessment:

- Drilling of 20 boreholes to depths of up to 15.65 m.
- Assessment of geotechnical issues affecting the proposed development.
- Geotechnical engineering parameters for concept foundation design (by others).
- Geotechnical parameters for pavement design (by others).

The objective of this work is to assist CCC develop their understanding of the geotechnical implications and constraints on developing the proposed facility on this site, and to assess the likely extra costs that may be incurred when compared to land underlain by relatively competent natural ground.

2 Site Description

2.1 General

Kyle Park is located at 197 Waterloo Road, which is approximately 9 km west of the Christchurch Central Business District. The 8.7 hectare site is bounded by:

- Waterloo Road, residential housing and Hornby Primary School to the north.
- Residential housing to the west.
- Rail lines, Denton Park, and "The Hub" shopping centre to the south.
- Varied commercial / industrial properties along Smarts Road to the east.

The site comprises two property parcels with legal titles of Lot 1 DP 78681 and Lot 2 DP 34558.

2.2 Current site usage

Kyle Park currently hosts various features and uses comprising:

- A pocket of trees surrounding a grassed bank adjacent to the western boundary.
- A stormwater retention pond that is surrounded with vegetation.
- Grassed recreational areas with sporadic trees and a BMX track in the central portion.
- Meandering footpaths that converge at the south into a pedestrian railway underpass link to Denton Park.
- Sports fields and grassed recreational areas with sporadic trees in the eastern portion.

2.3 Topography

The topography of the area surrounding the site is essentially flat. The ground beneath the site itself has been heavily modified over several decades and comprises a mixture of flat areas (sports fields), undulating terrain (BMX track and area surrounding the stormwater retention pond) and terraces / embankments.

The central area of the site is relatively flat and lies at an elevation of approximately 30 mLVD. The stormwater retention pond lies at approximately 27 mLVD, while the eastern end of the site lies at approximately 28 mLVD. The stormwater retention pond is bounded to the west and south by a raised embankment walkway, and to the north and east by grassed embankments (refer Photograph 1 in Appendix A). Generally, the site boundaries along Waterloo Road, Smarts Road and the rail lines comprise grassed embankments (Photograph 2, Appendix A).

2.4 Geological setting

The published geology¹ of the area indicates that the site is underlain by Holocene-age (less than 10,000 years old) alluvial gravel, sand and silt of historic Waimakariri River flood channels. This is collectively referred to as the Yaldhurst Member of the Springston Formation. There are no currently known or mapped earthquake fault lines in the vicinity of the site. Recent experience has highlighted that Christchurch lies within a seismically active area and earthquake hazard will be one of the main development considerations, from an engineering design perspective.

¹ Brown, L.J., Weeber, J.H. 1992: Geology of the Christchurch Urban Area. Institute of Geological & Nuclear Sciences Geological Map 1. Scale 1:25 000.

3 Ground and Groundwater Conditions

3.1 Geotechnical investigations

The field investigations were carried out between 19 September and 11 October 2018 and comprised 20 boreholes (labelled BH100 to BH120) to a maximum depth of 15.5 m below the existing ground surface, with Standard Penetration Tests (SPTs) at 1.5 m depth increments. The boreholes were drilled by Prodrill Limited under the direction of T+T using sonic drilling methodology with a water flush. BH113 was terminated at 7.6 m depth after refusal on a buried metal object.

The investigation locations are shown on Figure A1 in Appendix A. The borehole locations were surveyed by CCC to record their reduced level along with northing and easting coordinates (see Figure A2, Appendix A). Summary borehole logs are provided in Appendix C.

3.2 Geotechnical model

The geotechnical model for the site has been interpreted using the geotechnical investigation information collected as part of this commission and is summarised in Table 1 (below). In the development area the lowest elevation where landfill materials were observed in the boreholes varies from 25.8 mLVD to 28.6 mLVD, with an average elevation of 26.5 mLVD. Existing ground level is approximately 36 mLVD. Further discussion of each of the main soil layers is provided thereafter.

Soil layer	Extent of layer (typical)	Description	SPT N values
Capping materials	0 – 0.7 m	Sandy silt with some gravel	-
Landfill	0.7 – 10 m	Silt, sand, gravel, waste, organics	0 to 25
Dense gravel	10 m +	Sandy, dense to very dense gravel with some silt	> 50

Table 1 – Interpreted geotechnical model

3.2.1 Capping materials

Capping materials comprising sandy silt with variable quantities of gravel with fibrous organics and rootlets, all of which were encountered in the boreholes across the site. These materials ranged in thickness from 0.4 to 0.9 m. They generally graded into the underlying landfill materials with an increasing man-made material content and presence of organic materials rather than exhibiting a distinct change of strata. No evidence was observed of a geotextile fabric separating the capping materials from the landfill material.

3.2.2 Landfill

Landfill materials were encountered in all boreholes and exhibited highly variable content both laterally and vertically. The landfill materials comprised of a variable matrix of silt, sand and gravel with differing quantities of man-made materials and/or waste including:

- Asbestos Containing Material (ACM) including asbestos cement sheet fragments.
- Ash.
- Brick and concrete.
- Ceramic.
- Glass.

- Leather (including parts of a child's shoe).
- Paper.
- Plastic, including food wrapping.
- Roots, wood and partly decomposed vegetative matter.
- Rubber/tyre.
- Sawdust.
- Shell.
- Wire and metal.

Photographs 3 to 6 inclusive (Appendix B) illustrate some of the landfill materials observed and sampled across the site. In some instances it was noted that the vegetative/organic content of the landfill materials was high, in the approximate order of 50% by volume of material sampled.

3.2.3 Dense gravel layer

Natural strata were encountered in all boreholes (with the exception of BH113) and comprised of sandy fine to coarse gravels with minor cobbles and a trace of silt. SPT blow counts were greater than 50.

3.3 Groundwater

Groundwater was encountered in all boreholes (except for Borehole 113) at depths ranging from 9.2 to 11.3 m below the existing ground surface (approximately 29 mLVD and 24.5 mLVD).

With the exception of BH102 and BH106, groundwater was encountered within the underlying natural strata. For these two boreholes, the groundwater was encountered within landfill materials and within 1 m of the change of strata to the underlying natural materials.

4 Geotechnical Issues Identified

Table 2 (below) summarises the identified geotechnical engineering issues associated with the proposed development at this site. The main issues are the relatively low bearing capacity and potentially large consolidation settlements associated with the landfill. If not addressed appropriately in design these issues could result in damage to the proposed building and car park (and associated infrastructure). These considerations are not dissimilar to development on other sites where uncontrolled fill is present, e.g. ports and harbours and engineering solutions proposed to manage them would be relatively common.

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Issue	Comments	Proposed mitigation measures
Landfill materials	Low bearing capacity for shallow foundations. Large consolidation settlements which could lead to building damage and damage to carpark pavement, and other buried services.	Pile foundations driven through to the natural gravel layer.
Pile design – negative skin friction	Negative skin friction resulting in increased vertical downward loads on the piles is expected to occur due to settlement of the landfill materials over time.	Negative skin friction loads have been estimated and will need to be included in the pile design, with piles appropriately founded within the underlying dense naturally occurring gravelly soil.
Pile construction – refusal on landfill materials	Piles may refuse on obstructions or hard layers within landfill. The presence of obstructions other than at the location of BH113 cannot be discounted.	Use steel piles, with a large section size (for example 310UC158). Allow for some pile re-driving and re- positioning.
Noise and vibration associated with driven piles	May cause a nuisance during construction. Could result in damage to neighbouring structures – but not expected at this site.	Engagement with neighbouring properties. Vibration monitoring. Pre-construction dilapidation surveys. Pre-drilling the first 1-2 m could be contemplated.
Pile corrosion	Landfill material may accelerate corrosion of steel piles.	Epoxy coating. Use heavy steel section to accommodate a sacrificial corrosion allowance.
Car park – pavement damage	Ongoing landfill settlements may result in damage to the car park, for example resulting in cracking, potholes.	Place hardfill above landfill to build up to street level. Use thicker subbase to increase pavement strength. Use thicker asphalt layer to increase resilience. Create final landforms that facilitates surface drainage under gravity for stormwater (rather than below ground elements such as pipelines). Increase drainage grades to allow for some settlement.

Table 2 – Geotechnical issues identified

5 Foundation Assessment

5.1 Foundation options

A number of foundations options were considered for the proposed development, including:

- Piles these are recommended as they transfer the building loads through the landfill material into the dense gravel layer below.
- Shallow foundations not recommended due to the low bearing capacity of the landfill material and the potential for ongoing consolidation settlements (although re-levellable shallow foundations could be considered if Council are prepared to consider a higher degree of tolerable building disruption and periodic future repair/relevelling).
- Ground improvement methods such as dynamic compaction, stone columns or soil mixing may be feasible. However due to the relatively high organic content and the 10 m+ depth of the landfill materials the foundation performance of ground improvement methods is considered to be less certain than piles.

On the basis of the above high-level assessment we propose that pile foundations be considered for this development, which are discussed below.

Based on the high proportion of organic materials in the landfill, the potential for landfill gas generation cannot be discounted. The inclusion of protection measures in foundation selection and design to prevent landfill gas migration into the Centre will be required, depending upon the findings of a landfill gas investigation and assessment, which is discussed further in the accompanying T+T ground contamination report. Piling through landfill materials and inclusion of landfill gas protection measures in the construction of the centre are relatively commonplace in New Zealand.

5.2 Pile foundations

5.2.1 Pile type

Driven piles are the recommended pile option for the proposed development. Other pile types, such as bored piles or CFA piles, are not expected to be as economical due to the potential costs associated with spoil management. Screw piles may also be considered, but experience suggests that these are more likely to encounter refusal on obstructions within the landfill.

Steel H-piles are expected to be the most feasible option since they should be drivable through some potential hard layers or obstructions within the landfill. A heavy steel section, such as a 310UC158, is expected to be suitable.

5.2.2 Pile design parameters and capacity

Initial pile analysis indicates that an ultimate limit state (ULS) capacity of 1300 kN can be used for concept design for an 18 m long, 310UC158 steel pile driven to effective refusal into the dense gravel layer. The ULS capacity is based on the following:

• Geotechnical ultimate capacity (R_{drive}) of 2600 kN may be assumed for 18 m long, 310UC158 steel piles driven to effective refusal.

- Negative skin friction load (NSF) of 750 kN, caused by potential settlement of the landfill material over time. Research² suggests that NSF in landfill materials can vary between 5 to 30 kPa. Taking a conservative value of 40 kPa, and including a skin friction of 100 kPa in a 2 m thick layer of compacted hardfill placed on top of the landfill material, results in a NSF of 750 kPa.
- A strength reduction factor of $\phi_g = 0.7$. This strength reduction factors assume that 10% of all piles are proven by dynamic pile testing (PDA).
- Using the relationship below, the available ultimate limit state capacity (ULS) can be taken as 1300 kN.

$$R_{Drive} = \frac{ULS}{\varphi_g} + NSF$$

5.2.3 Durability

Corrosion of the steel will need to be considered in the pile design, as landfill material presents special circumstances around durability due to the presence of certain aggressive acids (both organic and inorganic), salts and solvents, which can chemically attack steel. A high performance epoxy coating should be applied to the piles to increase the durability. Alternatively, a corrosion allowance could be incorporated into the design of the pile cross-section. The Australian Piling Code, AS 2159 (2009), recommends that a uniform corrosion allowance of 0.04 - 0.1 mm/year for untreated steel should be adopted.

5.2.4 Construction considerations

Pile driving can cause noise and vibration that may affect the surrounding community. Noise and vibration effects decrease rapidly with distance from the piling rig, and given that there are no close neighbours it is expected that these effects can be effectively managed. Mitigation options could include:

- Engagement with neighbouring properties.
- Vibration monitoring to ensure vibration levels are below target values appropriately set to limit potential damage to structures.
- Pre-construction dilapidation surveys.
- Pre-drilling the first 1-2 m could also be contemplated.

Due to the potential to encounter unplanned obstructions within the landfill material, we recommend allowing an extra 10% to the piling costs to allow for re-driving and/or re-positioning of piles.

5.3 Seismic site subsoil class

We have assessed the soil profile at the site in accordance with Section 3.1.3 of NZS1170.5:2004. While the exact depth to rock is not known, deep groundwater bores in the area indicate in excess of 100 m thickness of interbedded silts, sands and gravels. The site subsoil class is therefore assessed to be Class D – Deep or Soft Soil.

² Bouazza & Kavazanjiam, State of the Art Paper, Construction on Old Landfills, 2nd Australian/New Zealand Conference on Environmental Geotechnics, Newcastle, November 2001.

5.4 Liquefaction assessment

Ground disruption at or close to the ground surface due to liquefaction is not expected to occur at this site. This is due to the 10 m depth to groundwater, the soils below the groundwater table comprising dense gravels that are not susceptible to liquefaction and the absence of ground surface disruption at the site following the 2010-2011 Canterbury Earthquakes.

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6 Pavement Design

Consolidation and settlement of landfill material, either due to decomposition of organic material within the fill and/or under traffic loads, may occur that could damage overlying pavement and buried pipelines.

We understand that the proposed car park area will be built up from the existing ground level (approximately 36 mLVD) by approximately 2 m to be consistent with the adjacent street level (approximately 38 mLVD). If a well-compacted engineered hardfill is used to do this it will enhance the pavement subgrade. Therefore the pavement requirements are not expected to be a significant extra/over cost compared to a non-landfill site.

A concept pavement detail (subject to further detailed design and confirmation of anticipated traffic loading) could comprise:

- 40 mm NZTA: M10/AC14 layer, overlying
- 150 mm NZTA: M/4 AP40 layer, overlying
- 2000 mm compacted AP65 hardfill overlying the landfill capping layer.

The carpark drainage should be designed to reduce the need for buried drainage pipes, with the goal of reducing future maintenance requirements. This can be achieved by creating a final landform such that surface gravity drainage is used to direct surface water runoff to the required collection points prior to discharge off the site. This may involve some additional filling. However, given that the site will be filled anyway, there is not expected to be any significant extra/over costs compared to a non-landfill site.

7 Further Work

The following further geotechnical work is proposed based on piled building foundations:

Developed Design Services

• Specific analysis in conjunction with the project structural engineer to develop the soilstructure interaction model and specifically size the foundations.

Detailed Design Services

- Prepare a detailed design report, including design of landfill gas protection measures, if required.
- Prepare pile design and construction specification.
- Review and comment on geotechnical aspects of the structural engineering drawings and specifications.

Construction Phase Services

- Site monitoring and PDA testing during pile installation.
- Review earthworks placement and compaction.
- Prepare completion report presenting as built records for the geotechnical aspects of shallow foundations and piles.

Careful civil engineering design of the final site landform will be needed to facilitate surface water drainage and reduce the need for buried stormwater pipework. The civil engineering associated with the development will also need to consider potential differential movement at transitions between areas with different depths of underlying fill, and also at the entrance to buildings founded on piles.

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Applicability 8

This report has been prepared for the exclusive use of our client Christchurch City Council, with respect to the particular brief given to us and it may not be relied upon in other contexts or for any other purpose, or by any person other than our client, without our prior written agreement.

Recommendations and opinions in this report are based on data from a discrete number of boreholes. The nature and continuity of subsoil away from the borehole locations are inferred and it must be appreciated that actual conditions could vary from the assumed model.

Tonkin & Taylor Ltd

Report prepared by:

Hayden Bowen **Geotechnical Engineer**

Authorised for Tonkin & Taylor Ltd by:

. Gordon Ashby

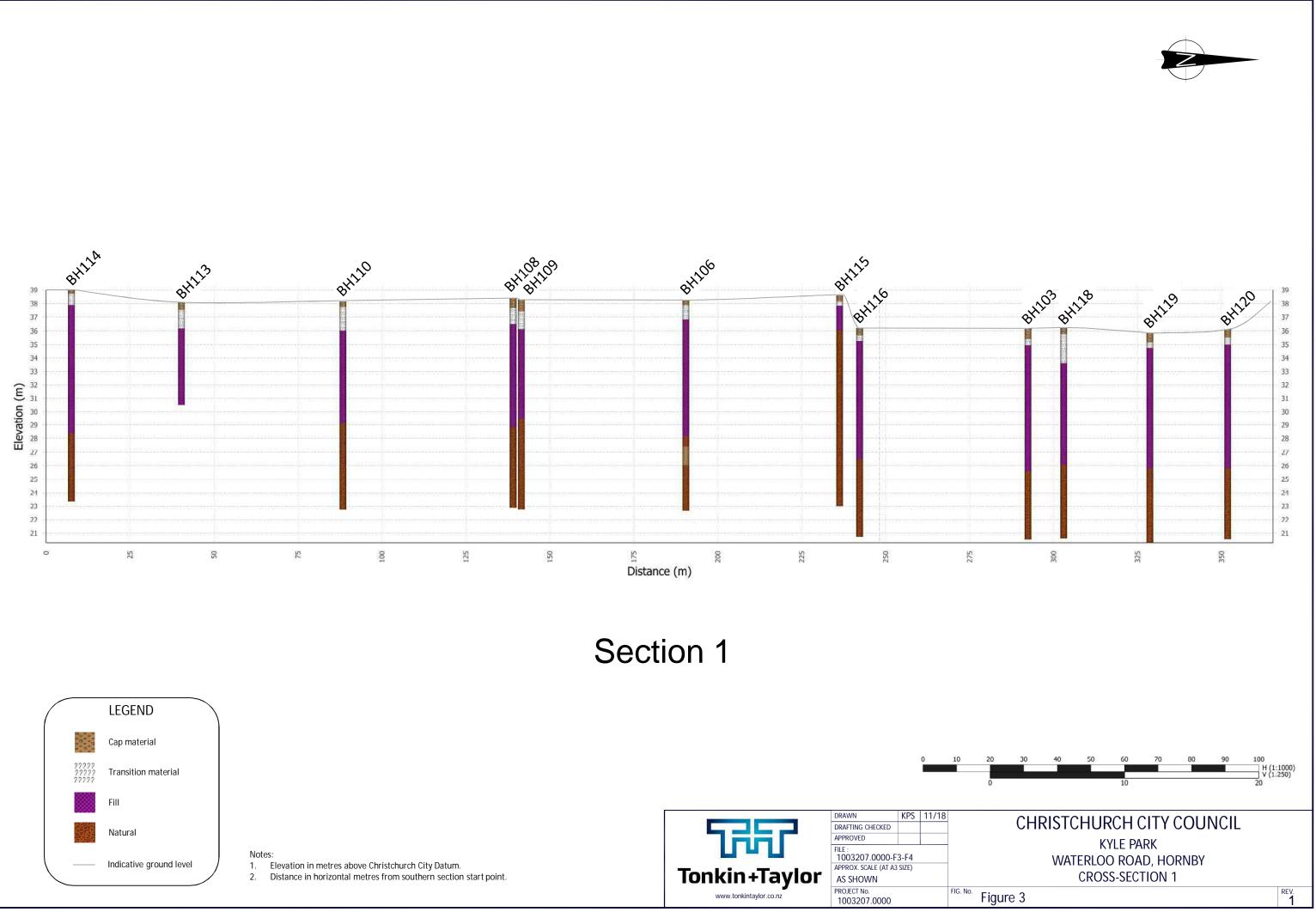
Project Director

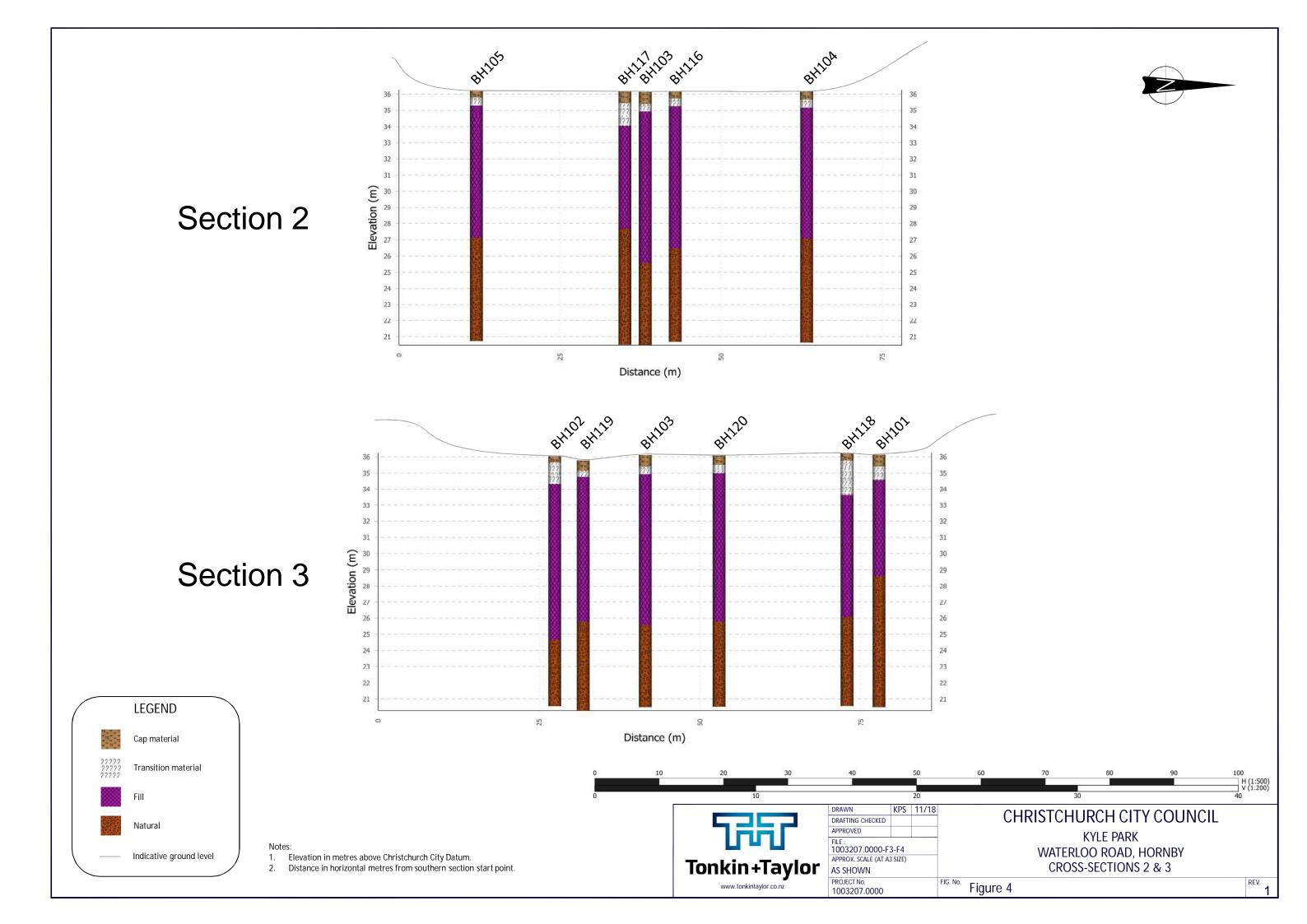
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Appendix A: Figures

- Figure 2 Site Plan
- Figure 3 Cross Section A-A'
- Figure 4 Cross Sections B-B' and C-C'





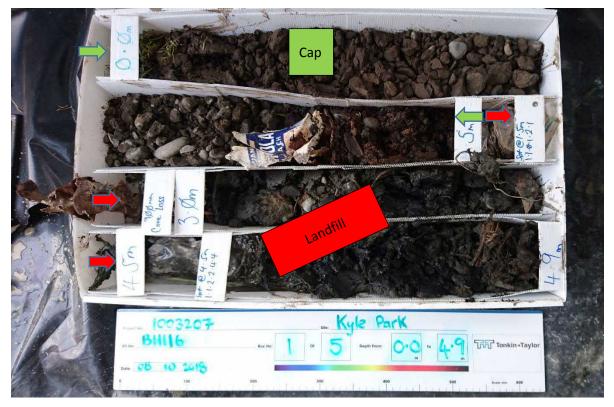




Photograph 1 –centre of the site looking south-west.



Photograph 2 – development area looking easterly (Waterloo Road left of frame).



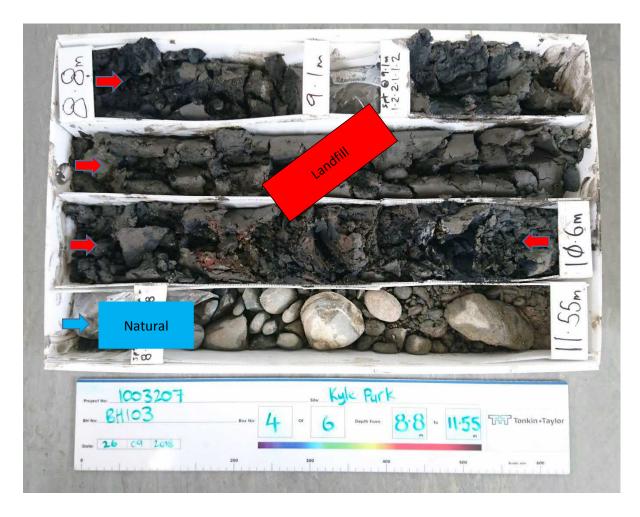
Photograph 3 – example of capping materials and their transition into landfill materials.



Photograph 4 – examples of landfill materials.



Photograph 5 – example of asbestos containing material (cement board materials) (Borehole 111 at 0.7 m bgl).



Photograph 6 - example of landfill and natural material change.



BOREHOLE No.: BH101

PROJECT: Kyle CO-ORDINATES: (NZTM2000)	517 517									DRIL	L TYPI	E: MS HOD:	1000	.,		bad, Hornby JOB No.: 1003207.0000 HOLE STARTED: 20/09/2018 HOLE FINISHED: 20/09/2018
R.L.:	36.1															
DATUM: GEOLOGICAL	CCE	J									L FLUI	D: WA	IER	FI		LOGGED BY: KPS CHECKED: HJB ERING DESCRIPTION
GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		25 50 FLUID LOSS (%) 75	WATER	CORE RECOVERY (%)	метнор	TESTS	SMAPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 50 100 (kPa) 200	1 5 20 20 50 8TRENGTH 50 60 60 60 8TRENGTH 250 250	20 60 DEFECT SPACING 200 (em) 2000 (em)	Description and Additional Observations
				66	PQ HFS			36			M M-W	F				Capping material: Sandy SILT with amorphous organics; dark brown. "Firm", moist, low plasticity, ve slow dilatancy. Contains trace rootlets; organic odour sand, fine to medium.
			-	73	PQ HFS P			35	2		W	-				Transition material: Sandy fine to coarse GRAVEL wi minor silt and trace cobbles; brown. Moist to wet, wel graded. Contains some coal (0.75 to 0.8m only); gravel, subangular to subrounded; sand, fine to medium. 1.0 to 1.5m - no recovery.
					۲ ۲			Ē								No SPT @ 1.5m (bouncing).
E8 9 9				100	SPT	7/8 6/3		33	3	\bigotimes		MD				Fill: organic and/or granular soils mixed with refuse. 2.6 to 3.0m - no recovery.
FILL			-	100	PQ HFS	2/1 N=12		32	4							
			-	100	SPT	2/1 4/4										For a general description of the landfill materials see the Geotechnical
ED. 989			-	100	PQ HFS	2/2 N=12		31	5							Assessment Report. Detailed field observations of the landfill material are available on request.
			-	100	SPT	1/3		- 30	6							
				100 1	PQ HFS S	2/2 3/3 N=10		29	7							
			-	100	SPT P	4/2 2/2 1/2			8			L				Silty sandy fine to coarse GRAVEL with trace cobble dark brownish grey. Loose, wet, well graded. Gravel, subangular to subrounded; sand, fine to coarse.
Ša				71	PQ HFS	N=7		28		0.0						8.1 to 8.3m - no recovery.
			-	100	SPT	13/13 10/12		27	9		M-W	VD				9.1m - grey; moist to wet, very dense.
Ē			20/09/2018 10.6 m bgl	100 1	PQ HFS S	10/12 16/12 for 65mm N>=50 Bouncing			10							
			▲ ^{20/}			7/5		26 			W-S	D				Sandy fine to coarse GRAVEL with minor silt and tra
NATURAL			-	100 100	PQ HFS SPT	6/6 14/18 N=34 Solid		25	11							cobbles; brown. Dense, wet to saturated, well grade Gravel, subangular to subrounded; sand, fine to coarse. 10.6 to 11.05m - no recovery from SPT; sample
			-			8/12		24	12			VD				obtained from overcore.
			-	100	SPT	16/18 16 for 55mm										12.2m - very dense. 12.2 to 12.56m - no recovery from SPT; sample obtained from overcore.
				100	PQ HFS	N>=50 Solid Bouncing		23	13							13.7 to 14.15m - no recovery from SPT; sample
				100	SPT	4/4 5/6 10/15		22	14							obtained from overcore.
				100	PQ HFS	N=36 Solid			15	0.00						
5				0	SPT	3/5 7/9		21		$\mathbf{\Sigma}$		D				15.2m - dense. 15.2 to 15.65m - no recovery from SPT.
						15/18 N=49 Solid		20	16							End of borehole at 15.65 m bgl (target depth).
COMMENTS: Hole Depth 15.65m								<u>F</u>		1						<u> </u>



BOREHOLE No.: BH102

DATUM: CC GEOLOGICAL UNT. GEOLOGICAL UNT. GRIERIC NAME. ORIGIN, MATERIAL COMPOSITION.	(%) SSO I UNIT SSO I U	WATER	80	PQ HFS METHOD CASING	TESTS	SAMPLES	Ê		DRILL	MEATHERING	D: WA		EN	-	LOGGED BY: KPS CHECKED: H. EERING DESCRIPTION
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, IMATERIAL COMPOSITION.	8월 FLUD LOSS (%)	WATER	80 CORE		TESTS	SAMPLES	Ē			HERING		E	EN	-	
GENERIC NAME. ORIGIN. MATERIAL COMPOSITION.	28 FLUD LOSS (%)	WATER	80 CORE		TESTS	SMAPLES	Ê	-		HERING		Æ		CING	
	1882 1		80			SA		DEPTH (m)	GRAPHIC LOG	MOISTURE WEAT	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)	COMPRESSIVE STRENGTH (MPa)	DEFECT SPACING (cm)	Description and Additional Observations
				<u> </u>				8	e Contraction	¥8 M M-W	S	20 20 20 20 20 20 20 20	50 50 100 50 50 50 50 50 50 50 50 50 50 50 50 5	2000	Capping material: SILT with some sand, amor organics; brown to dark brown. "Soft", moist, I plasticity, very slow dilatancy. Contains trace a organic odour; sand, fine to medium.
			$1 \ge 1$	SPT	2/1		35	1 -	$\overset{\otimes}{\sim}$		F-St				Transition material: Sandy fine to medium GR with some silt, brown to dark brown. "Loose", graded. Contains minor coal; organic odour; g
				PQ HFS S	2/1 0/0 N=3		34	2	$\otimes\!\!\!\!\otimes\!$						subangular to subrounded; sand, fine to coars 0.7m - sandy SILT with some gravel; brown. " firm", moist to wet, low plasticity, very slow dil 0.95m - thin rust layer.
				SPT F	4/4 6/5 17/7		- 33	3	\bigotimes						1.0m - "firm to stiff", trace glass. 1.2 to 1.5m - no recovery.
			6	PQ HFS	N=35		32	4	\mathbb{X}						 1.5 to 1.95m - no recovery from SPT; sample from overcore. 1.5m - gravelly, trace cobble. Fill: organic and/or granular soils mixed with r
-			59	PQ HFS			31	5	\bigotimes						2.1 to 3.0m - no recovery. 3.55 to 4.5m - no recovery. No SPT @ 4.5m (wood).
FILL				SPT	2/1 1/1		30	6	\approx						5.9 to 6.1m - no recovery.
				PQ HFS	1/1 N=4		29	7							7.04.7.05. 50.000000
NY X X 201				SPT B	1/1 1/1 1/1		28	8							7.2 to 7.6m - no recovery. 7.6 to 8.05m - no recovery from SPT; sample from overcore.
			100	PQ HFS	N=4			9 -							For a general description of the lan materials see the Geotechnical Assessment Report.
				FS SPT	1/1 1/1 0/2 for 65mm			-							Detailed field observations of the la material are available on request.
		20/09/2018 11.0 m bgl		SPT PQ HFS	N>=50 7/10 12/18		26	10	\bigotimes						10.1 to 10.4m - no recovery.
	_	•		PQ HFS S	20 for 65mm N>=50 Bouncing		25	11		w	VD				Sandy fine to coarse GRAVEL with minor silt
					17/33 for 75mm N>=50	\parallel	24	12							cobbles; brownish grey. Very dense, wet, wel Gravel, subangular to subrounded; sand, fine coarse.
NATURAL			100	PQ HFS	Solid Bouncing		23	13		W-S					 12.2 to 12.35m - no recovery from SPT; sampoblained from overcore. 12.7m - brown; wet to saturated.
				ь s	8/19 36/14 for 20mm N>=50	H	- 22	14							13.7 to 13.95m - no recovery from SPT; samp obtained from overcore.
				PQHFS	Solid ^{Bouncing} 7/13 19/31 for 75mm		21	15	0.0.0 0.0.0 0.0.0						
			0	SPT	N>=50 Solid Bouncing		20	16							15.2 to 15.5m - no recovery from SPT. End of borehole at 15.5 m bgl (target depth).



BOREHOLE No.: BH103

CO-ORDINATES:	51791									DRILI	TYPE	E: MS	1000			HOLE STARTED: 21/09/2018
(NZTM2000) R.L.:	15616)0 m	ιE						DRILI	MET	HOD:	SNC			HOLE FINISHED: 21/09/2018 DRILLED BY: ProDrill
DATUM:	36.14r CCD	11								DRILI	_ FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL														E	NGINE	ERING DESCRIPTION
GEOLOGICAL UNIT,																
GENERIC NAME, ORIGIN,											RING		IGTH	8	DEFECT SPACING (cm)	Description and
MATERIAL COMPOSITION.	9 (%) 9 (%)		RY (%)			TESTS					WEATHERING	NSITY N	SHEAR STRENGTH (KPa)	COMPRESSIVI STRENGTH (MPa)	(cm	Additional Observations
	(%) SSOT (%)	~	CORE RECOVERY (%)	8	0		ES		(E)	GRAPHIC LOG	MOISTURE	STRENGTH/DENSIT CLASSIFICATION	SHEA	000	ö	
	2 88	WATER	CORE	METHOD	CASING		SAMPLES	RL (m)	DEPTH (m)	GRAPI	MOIST		300 222 ⁽		200 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	
								30		\otimes	М	s				Capping material: SILT with some sand, amorphous organics; brown to dark brown. "Soft", moist, low to
			93	PQ HFS				Ē		\otimes						moderate plasticity, very slow dilatancy. Contains tra-
			0,	g				35	1 -	\otimes		L				rootlets; organic odour; sand, fine to medium. Transition material: Sandy fine to coarse GRAVEL wi
										\boxtimes						some silt, amorphous organics; dark brown. Loose,
			100	SPT		2/1 1/1		Ē		\otimes						moist, well graded; low to moderate plasticity, no dilatancy. Organic odour; gravel, subangular to
				ι. Υ]	1/1 N=4		34	2 -	XX	W-S					subrounded; sand, fine to medium.
F			71	PQ HFS				E		\otimes						Fill: organic and/or granular soils mixed with refuse. 1.4 to 1.5m - no recovery.
								Ē	3 -	\bowtie						2.0m - wet to saturated.
- Xog			100	SPT		2/1 0/1		- 33 -	2	\otimes						2.7 to 3.0m - no recovery.
				S		2/1 N=4		E		\otimes						
			100	PQ HFS				32	4 -	\otimes						
				-				ŧ		\times						
								Ē	-	$ \times $						No SPT @ 4.5m (steel). 4.5 to 5.2m - no recovery.
FILL			56	PQ HFS				31	5 -	\sim						
				Pa				E		\bigotimes						
5 6								É ac	6	\otimes						For a general description of the landfill
EG. 9 6 7 7 8			100	SPT		13/2 2/1		- 30		\otimes						materials see the Geotechnical Assessment Report.
					1	1/1 N=5		Ē		\otimes						Detailed field observations of the landfill
			100	PQ HFS				29	7 -	\otimes						material are available on request.
								Ē		\otimes						
=			100	SPT		2/2 1/1		Ē	Q -	\otimes						
0 0 0				s]	1/1 N=4		28	U	\otimes						
2 Y Y Y			100	PQ HFS				F		\otimes						
						4.16		27	9 -	\otimes						
			100	SPT		1/2 2/1		E 2'		\otimes						
				ŝ		1/2 for 65mm		Ē		\bigotimes						
		21/09/2018 10 9 m hol	100	PQ HFS		N>=50		26	10 -	\otimes						
		21/05	e.0			8/10		Ē		X		D				Sandy fine to coarse GRAVEL with minor cobbles an
		▼	82	SPT		12/8 8/8		Ē	11 -	<u> </u>		-				trace silt; brownish grey. Dense, wet to saturated, we
E 0				S		N=28		25								graded. Gravel, subangular to subrounded; sand, fine to coarse.
_			100	PQ HFS				Ē								
						8/9		24	12 -							
			33	SPT		125/10 18/10		E		$\dot{o}^{0}_{i}\dot{o}^{i}_{j}$	S	VD				12.2m - cobbles absent; brown. Very dense, saturate 12.2 to 12.65m - no recovery from SPT; sample
NATURAL				S		for 70mm N>=50		È	13 -	0.00						obtained from overcore.
NATURAL			100	PQ HFS		Bouncing		23	10	စံုံရံ						13.7 to 14.15m - no recovery from SPT; sample
						7/13 14/12		É								obtained from overcore.
			55	SPT		14/12 14/10 for 70mm		- 22	14 -	ð.°ð						
				FS		N>=50 Solid		Ē		o.o°d						
E			100	PQ HFS		Bouncing		Ē	-	0.00 0.00						
						6/11 15/15		21	15 -	k°.7						15.2 to 15.63m - no recovery from SPT
× 0 1			0	SPT		10/10 for 50mm		<u> </u>						<u></u>		15.2 to 15.63m - no recovery from SPT.
						N>=50 Solid		É ac	16 -							End of borehole at 15.63 m bgl (target depth).
						Bouncing		20								
COMMENTS:																



BOREHOLE No.: BH104

PROJECT: Kyle														, wa	10110		Dad, Hornby JOB No.: 1003207.0000
CO-ORDINATES: (NZTM2000)	51792 15615										L TYP						HOLE STARTED: 26/09/2018 HOLE FINISHED: 26/09/2018
R.L.:	36.17	m								DRIL	L MET	HOD:	SNC				DRILLED BY: ProDrill
DATUM:	CCD									DRIL	L FLUI	D: WA	TER				LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL															ΕN	GINE	ERING DESCRIPTION
GEOLOGICAL UNIT, GENERIC NAME,																G	
ORIGIN, MATERIAL COMPOSITION.											WEATHERING		ENGTH	SIVE		DEFECT SPACING (cm)	Description and
	(%) 55		VERY (%			TESTS				0		ION ION	SHEAR STRENGTH (KPa)	COMPRESSIV	(MPa	DEFECT (c	Additional Observations
	22 HIDLOSS (%)		CORE RECOVERY (%)	METHOD	5N		SAMPLES	Ê	DEPTH (m)	GRAPHIC LOG	MOISTURE	STRENGTH/DENSIT CLASSIFICATION	SHE	0			
	88	WATED	N NOO	MET	CASING		SAM	RL (m)	DEP		NON M	S STR	865833 965833		250	2000 1 2000 1 2000 2000	
				0				- 36		\otimes							Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown.
			08 0	2 HFS				E		$\overset{\times}{\triangleright}$							"Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to
				, a				35	1	***							medium; gravel, fine to medium, subangular to subrounded.
			-		-	2/0		Ē		***							Transition material: SILT with some sand and trace
			44	F F		0/1 1/1	_	Ę	2	\boxtimes	M-W						gravel; brown mottled greyish brown. "Soft", moist, plasticity, very slow dilatancy. Contains trace red
E			-	HFS -		N=3		- 34	-	\otimes	IVI-VV						plastic; sand, fine to medium; gravel, fine to mediur
			61	, la				Ę		\mathbb{N}	1						subangular to subrounded. 0.6 to 0.9m - no recovery.
<u>8</u>			100	SPT	1	3/3		33	3	\mathbf{k}							Fill: organic and/or granular soils mixed with refuse
			7			2/2 1/2		Ē		\otimes							1.7 to 1.95m - no recovery.
			61	HFS		N=7		Ē	Λ.	\otimes							2.1m - moist to wet. 2.6 to 3.0m - no recovery.
				, a				32	4	\otimes							
FILL			100	SPT	1	1/3 10/40		F		\otimes							
_				0		for 70mm N>=50		- 31	5	\mathbf{x}							4.8 to 5.0m - no recovery.
m0.9-01			88	PQ HFS		Bouncing		Ē		\otimes							
Box 2, 30-5,01				۱A				Ē	_	\otimes							For a general description of the landfill materials see the Geotechnical
—					1			- 30	6	\otimes							Assessment Report.
				S S				Ē		***							Detailed field observations of the landfil material are available on request.
			100	PQ HFS				Ē	7	\otimes							
								29		\otimes							
			c	sPT '	1	2/1 0/0		Ē		\sim	1						7.6 to 8.05m - no recovery in SPT.
			\vdash			1/2 N=3		28	8	\mathbf{k}							
Ę			47	PQ HFS		C-11		Ē									8.55 to 9.1m - no recovery.
26-0-0-2								Ē	9								0.00 to 9. mi - no recovery.
ю́хоод			100	SPT		3/2 3/4		- 27	-	فكأذ	w-s	MD					Sandy fine to coarse GRAVEL with minor cobbles a trace silt; bluish grey. Medium dense, wet to satural
		26/09/2018	bg u		1	5/5 N=17		Ē		ခိုင်							well graded. Gravel, subangular to subrounded; sai
		26/0	10.4	PQ HFS		Solid		26	10	÷.	s	L					fine to coarse. 9.1 to 9.55m - no recovery from SPT; sample obtain
			Ľ			2/1		Ē		åQ 0.00							from overcore.
			44	SPT 1		2/2 2/2		Ē	11]						10.0m - reddish brown; saturated, loose. 10.8 to 11.05m - no recovery.
				ι. Υ		N=8		25		¢;oo							
-			100	PQ HFS				É									12.0m - trace to minor silt, trace cobbles; brownish grey.
						9/12		24	12	¢,°°							
			100	SPT		10/10 12/6		E		$\dot{o}^{0}_{0}\dot{o}$		D					12.2m - dense.
<u> </u>				ŝ		N=38		Ę	13	0.00							12.7m - sandy, trace to minor silt; brown.
			100	PQ HFS				23	15	ۇ ⁰ ،							
			-			7/13 19/11		Ē		0. 3.8°	1	VD					12 Zma supervision a
			100	SPT		19/11 10/10 N>=50		22	14								13.7m - very dense.
				HFS		11/-50											
.7-15.6m			100	PQ H		9/17		Ē		òòò							
-7.21,65					-	21/23 6		21	15	₽.°°	1						15.2 to 15.1m - no recovery from SPT.
8			+	- is	\square	for 70mm N>=50		Ē		\downarrow							End of borehole @ 15.51m bgl (target depth).
						Solid Bouncing		É	16	-							
								20		-							
COMMENTS:																	
ole Depth 15.51m																	



BOREHOLE No.: BH105

CO-ORDINATES: (NZTM2000) R.L.: DATUM:	Park 51791 15615 36.34i CCD	61.0								DRIL	L MET	e: MS Hod: D: WA	SNC			HOLE STARTED: 26/09/2018 HOLE FINISHED: 26/09/2018 DRILLED BY: ProDrill LOGGED BY: KPS CHECKED: HJB
								1	-					E		
GENERIC NAME. ORIGIN, MATERIAL COMPOSITION.	28 FLUID LOSS (%)	75 VATER	CORE RECOVERY (%)	МЕТНОD	CASING	TESTS	SMIPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIF/CATION	10 26 50 SHEAR STRENGTH 100 (kPa) 200	1 5 20 20 5 20 50 50 60 60 700 87RENGTH 100 850 250	20 60 80 80 80 800 800 800 800	Description and Additional Observations
			93	PQ HFS				36	1		М	L				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
			100	SPT		2/1 0/0				\otimes						0.35m - yellowish brown. Transition material: sandy fine to coarse GRAVEL w
			100	PQ HFS		1/0 N=1		34	2	\bigotimes						minor to some silt, amorphous organics; dark brown "Loose", moist, well graded. Contains trace glass, white paint/plaster chips; organic odour; gravel, subangular to subrounded; sand, fine to medium.
<u> </u>			100	SPT		1/1 3/2		- 33	3	\otimes						Fill: organic and/or granular soils mixed with refuse.
			100	PQ HFS		2/2 N=9			4							1.4 to 1.5m - no recovery.
FILL			100	SPT		3/3 2/2		32		\otimes						For a general description of the landfill materials see the Geotechnical
			100	PQ HFS		2/2 for 70mm N>=50		31	5	\bigotimes						Assessment Report. Detailed field observations of the landfill material are available on request.
-			100	SPT		1/2 2/2		30	6	\bigotimes						
			100	ι. Ω		1/2 N=7		- 29	7		w					6.5m - wet.
			100	SPT		4/4 3/7			0	\bigotimes						
			100	PQ HFS		3/1 N=14		28	0	\bigotimes						
		018	100	SPT		3/4 4/16		- 27	9 -		W-S	VD				Sandy fine to coarse GRAVEL with minor silt and tr cobbles; brown. Very dense, wet to saturated, well
		26/09/2018	100	PQ HFS		20/10 for 10mm N>=50 Bouncing		26	10		S	-				graded. Gravel, subangular to subrounded; sand, fi to coarse. 9.7m - greyish brown; saturated.
			100	PQ HFS		20/30 for 70mm N>=50 Solid _{Bouncing}		25	11							10.6 to 11.05m - no recovery from SPT; sample obtained from overcore. 10.8m - trace silt; bluish grey.
NATURAL			100	SPT		12/19 25/25		- 24	12	0.0 00 0.0						12.0m - brownish grey. 12.2m - trace to minor silt; brown.
			100	PQ HFS		for 70mm N>=50 Solid Bouncing			13							12.2 to 12.35m - no recovery from SPT; sample obtained from overcore.
			100			11/17 18/18 14 for 60mm		23	14							13.7 to 14.06m - no recovery from SPT; sample obtained from overcore.
			100	PQ HFS		N>=50 Solid Bouncing 13/21		22	15 ⁻							
			0	SPT		27/23 for 65mm N>=50 Solid Bouncing		21	16							15.2 to 15.49m - no recovery from SPT. End of borehole @15.49m bgl (target depth).



BOREHOLE No.: BH106

CO-ORDINATES:	5179										DRIL	L TYPI	E: MS	1000			HOLE STARTED: 27/09/2018
(NZTM2000) R.L.:	156 ⁻ 38.2) mE	Ξ						DRIL	L MET	HOD:	SNC			HOLE FINISHED: 27/09/2018 DRILLED BY: ProDrill
R.L.: DATUM:	38.2 CCE										DRIL	L FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL															E١	NGINE	EERING DESCRIPTION
GEOLOGICAL UNIT,																	
GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		(%)		(%) ۲۶			TESTS					WEATHERING	N N	SHEAR STRENGTH (KPa)	COMPRESSIVE STRENGTH (MPa)	DEFECT SPACING (am)	Description and Additional Observations
		50 FLUID LOSS (%)	WATER	CORE RECOVERY (%)	METHOD	CASING		SAMPLES	RL (m)	DEPTH (m)	SRAPHIC LOG		STRENGTH/DENSITY CLASSIFICATION	10 255 SHEAF 500 200			
			-			-			38		\bigotimes	м	S				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy.
				100	PQ HFS				37	1 -							Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
E 92				100	SPT		3/2 2/2 2/2			2 -							Transition material: SILT with some sand and trace gravel; brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace brick; sand, fine to medium
Box 1, 0.0-2.5m				71	PQ HFS		N=8		36	2	\bigotimes						gravel, fine to medium, subangular to subrounded.
				100	SPT F		2/1 3/2		35	3 -	\bigotimes						Fill: organic and/or granular soils mixed with refuse. 2.7 to 3.0m - no recovery.
				76	PQ HFS		2/2 N=9			4 -	\bigotimes						
				100	SPT P		1/2 2/1		34		\bigotimes						For a general description of the landfill materials see the Geotechnical Assessment Report.
				100	PQ HFS		0/1 N=4		33	5 -							Detailed field observations of the landfill material are available on request.
				100	SPT		2/4 4/4 6/4		32	6 -	\bigotimes						
E. Ŷ				100	PQ HFS		0/4 N=18		31	7 -							
196X 54 94 94 94 94 94 94 94 94 94 94 94 94 94			2	100	SPT		2/2 6/4 3/3		- 30	8 -	\bigotimes						
			4 2 / 109/2014 9.2 m bgl 9.2 m bgl	100	PQ HFS		N=16			0 -	\bigotimes						
			V	100	SPT		9/9 11/7 5/5		29	9	\bigotimes						
Ex.				80	PQ HFS		N=28		28	10 -	***	s	L				9.9 to 10.1m - no recovery. Fine to coarse GRAVEL with trace sand and silt;
- 0.5 4. 20 - 0.5				100	SPT		10/6 3/3 2/2 N=10			11 -							brownish grey. Loose, saturated, well graded. Grave subangular to subrounded; sand, fine to coarse. 10.3m - sandy.
				4	PQ HFS				27								Silty fine to medium SAND; grey. Loose, saturated, poorly graded.
				100	SPT		12/12 15/20 15 for 20mm		26	12 -	å.º.ċ		VD				11.1 to 12.2m - no recovery. Sandy fine to coarse GRAVEL with trace cobbles an silt; brownish grey. Very dense, saturated, well grade
NATURAL				100	PQ HFS		N>=50 Bouncing		25	13 -							Gravel, subangular to subrounded; sand, fine to coarse. 12.2 to 12.52m - no recovery from SPT; sample
E / 4				100	SPT		4/5 8/12 12/12 N=44			14 -			D				obtained from overcore. 13.2m - minor silt; brown. 13.7m - dense.
m/xF-801 cc xool				100	PQ HFS		Solid 6/8		24								13.7 to 14.15m - no recovery from SPT; sample obtained from overcore.
- 14-7				0	SPT		15/15 18/2 for 5mm N>=50		23	15 -	×		VD				15.2m - very dense. 15.2 to 15.58m - no recovery from SPT.
							Solid Bouncing		22	16 -	-						End of borehole @ 15.58m bgl (target depth).
COMMENTS:									r			I	I				1



BOREHOLE No.: BH107

	38.61n CCD		CORE RECOVERY (%)	METHOD	TESTS						HOD: D: WA				DRILLED BY: ProDrill LOGGED BY: KPS CHECKED: HJB
GECOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION		WATER		ЕТНОР	TESTS						D. W/		ENC		LOGGED DT. KF3 CHECKED. HJD
GEOLOGICAL LINIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.	(%) sson ann 2 182	WATER		ЕТНОD	TESTS						_		ENG	SINE	ERING DESCRIPTION
FILL	1382	WAT		5	CASING	e MIDI EC	e fe	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (KPa)	COMPRESSIVE STRENGTH (MPa)	DEFECT SPACING (cm)	Description and Additional Observations
<u>s</u> FILL			100	PQ HFS ME	85	WS	(iii) 121 111111 111111 111111	1 -	CLARK CLARK	M W W	S L	26 26 100 100 100	20 10 10 10 10 10 10 10 10 10 1	2000	Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
<u>§</u> FILL			100	SPT	2/3 3/2		37		\otimes						Transition material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown
			06	PQ HFS	2/1 N=8		36	2 -							mottled light grey and orange. "Soft", moist, low plasticity, very slow dilatancy. Contains trace brick; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
			100	SPT	3/3 2/2			3 -	Ŵ						0.65m - organic sandy fine to coarse GRAVEL with minor to some silt; brown to dark brown. "Loose", w
			85	PQ HFS	1/2 N=7		35	4 -							well graded. Contains trace brick; organic odour; gravel, angular to subrounded; sand, fine to coarse organics, amorphous. 0.9m - trace white paint/plaster chips. 1.05m - light grey and orange bands.
							34	_	\otimes						Fill: organic and/or granular soils mixed with refuse
			75	PQ HFS			-	5		М	VD				2.9 to 3.0m - no recovery. 4.35 to 4.5m - no recovery. No SPT at 4.5m (wood).
			100	SPT	15/15 12/12		- 33	6							Sandy fine to coarse GRAVEL with minor to some and amorphous organics; dark brownish grey. Ver dense, moist, well graded. Gravel, subangular to subrounded; sand, fine to coarse.
			100	PQ HFS	14/12 N>=50 Bouncing		- 32	7 -	0.00 0.00 0.00	w	-				5.2m - trace silt, organics absent; grey. 5.3m - minor cobbles. 5.7 to 6.1m - no recovery.
			66	SPT F	14/16 16/14		31								 7.0m - trace cobbles; grey, wet. 7.6 to 7.85m - no recovery from SPT; 200mm same
			100	PQ HFS	12/8 for 70mm N>=50 Solid		30	8 -							obtained from overcore. 7.85 to 8.0m - sand and silt absent.
		27/09/2018 10.1 m bgl	2	SPT	16/18 18/18 14			9 -							9.1 to 9.47m - no recovery from SPT; 170mm sam obtained from overcore.
NATURAL		▲ ^{27/0}	100	PQ HFS	for 65mm N>=50 Solid Bouncing		29	10 -							9.3m - minor silt. For a general description of the landfill
-			100	PQ HFS	24/26 for 75mm N>=50 Solid Bouncing		28	11 -							materials see the Geotechnical Assessment Report. Detailed field observations of the landfill material are available on request.
			100	HFS	30/20 for 70mm N>=50 Solid Bouncing		26	12 - 13 -							12.2 to 12.35m - no recovery from SPT; sample obtained from overcore.
			100	SPT PQ	10/10 14/12 14/10		25	14	0.00						13.7m - brown.
-			100 1	PQ HFS S	for 70mm N>=50 Solid Bouncing		24	14	0.0.0 0.0.0						13.7 to 14.15m - no recovery from SPT; sample obtained from overcore.
			0	SPT	18/27 32/18 for 40mm			15 -	°. C						15.2 to 15.47m - no recovery from SPT.
					N>=50 Solid Bouncing		23	16 -							End of borehole @ 15.47m bgl (target depth).



BOREHOLE No.: BH108

CO-ORDINATES:	517911														
(NZTM2000)	156148								DRIL	LIYP	E: MS	1000			HOLE STARTED: 02/10/2018 HOLE FINISHED: 02/10/2018
R.L.:	38.43m								DRIL	L MET	HOD:	SNC			DRILLED BY: ProDrill
DATUM:	CCD								DRIL	L FLU	ID: WA	TER			LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL													E١	IGINE	ERING DESCRIPTION
GEOLOGICAL UNIT,															
GENERIC NAME, ORIGIN,										ERING		NGTH	Э.H.	DEFECT SPACING (cm)	Description and
MATERIAL COMPOSITION.	S (%)		ERY (%)		TESTS					WEATHERING	NSITY	SHEAR STRENGTH (KPa)	COMPRESSIN STRENGTH (MPa)	EFECT S	Additional Observations
	28 80 75 75 75	~	CORE RECOVERY (%)	9	o		ES	Ē	GRAPHIC LOG	MOISTURE	STRENGTH/DENSITY CLASSIFICATION	SHEA	δ.o	ä	
	288 788	WATER	CORE	METHOD	CASING		SAMPLES RL (m)	DEPTH (m)	GRAPI	MOIST		10 25 20 20 20 20 20	- 1 5 - 20 - 20 - 100 - 250	88888	
							Ē			м	s				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown.
			100	PQ HFS			- 3	5							"Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to
			-	g			Ē	1							medium; gravel, fine to medium, subangular to
			,	ž	0/40	L	3	7							subrounded. 0.3m - brown mottled light yellowish brown.
					8/42 for 50r	nm	Ē		\otimes						Transition material: SILT with some sand and trace
			100	PQ HFS	N>=5 Bouncir		Ē	2							gravel; brown mottled light yellowish brown. "Soft", moist, low plasticity, very slow dilatancy. Contains
<u>i</u>			-	g			- 3	j.	XX	W-S					trace brick, white paint chips, and timber; sand, fine medium; gravel, fine to medium, subangular to
			0	$\left \right $	2/3			3	\rightarrow						subrounded.
			100	SPT	3/3		3	5	\otimes						Fill: organic and/or granular soils mixed with refuse 2.4m - wet to saturated.
			100	S L L	N=9		È		\otimes						2.7111 - WEL IO SALUIALEU.
			10	PQ HFS			Ē	4	\otimes						For a general description of the landfill
			100	SPT	2/1		- 3	4						$\begin{array}{c} 1 & 1 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 & 1 \\ 1 & 1 &$	materials see the Geotechnical
FILL			1	S	0/1 1/2			5							Assessment Report.
			82	PQ HFS	N=4		- 3	3							Detailed field observations of the landfill material are available on request.
			8	ğ			Ē		\otimes						
			100	SPT	1/1			6							5.9 to 6.1m - no recovery.
<u>i</u>			10	ŝ	3/5 9/3		3	2							
			14	PQ HFS	N=2)	Ē	7		1					6.7 to 7.6m - no recovery (rubbish blocking barrel). No SPT @ 7.6m.
			-	g			- 3	1							
							Ē		∞	Ì					
				FS			Ē	8		4					
			73	PQ HFS			- 3	D	\rightarrow						
							Ē	9]					8.7 to 9.1m - no recovery.
-			100	SPT	8/10 6/5		- 2	9	\otimes						
2)18)gl		ទួ	5/5 N=2	· [Ē		Š.	\$	VD				Sandy fine to coarse GRAVEL with minor cobbles a
3		02/10/2018 10.7 m bgl	100	PQ HFS			Ē	10	Å.						silt; brownish grey mottled orange. Very dense, we saturated, well graded. Gravel, subangular to
		▲ 10			13/1	3	2	В							subrounded; sand, fine to coarse. 9.7m - reddish orange.
			100	SPT	20/2: 8	2	-	11							9.8m - bluish grey.
			0	FS F	for 5m N>=5	0	- 2	7	a di ci						11.0m - grey.
			100	PQ HFS	Bouncir	g	Ē		စ္ႏိ						
			0	+	10/2			12	- Yo	ł					
NATURAL			100	8	26/14 for 5m	m	- 2	6	á chác chiến thể chiến thế chiến thể					$\begin{array}{c} 1 & 1 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 & 1 \\ 1 & 1 &$	12.2 to 12.43m - no recovery from SPT; sample obtained from overcore.
			100	PQ HFS	N>=5 Solic Bouncir	ı	Ē	13	စ္ႏိ						12.7m - trace silt; bluish grey.
-			1	g	Dounda	9	- 2		ģ						
			0	F	4/6 9/7			-	0. گل						13.7 to 14.15m - no recovery from SPT; sample
			100	SPT	11/1 N=4			14	La c						obtained from overcore.
_			100	LES	Solid		2	4	<u>نې</u>						14.0m - brown. 15.2 to 15.57m - no recovery from SPT.
			10	PQ HFS	5/7		È	15							
			0	SPT	14/1 20 for 65r		2		၀.၀ိ ကို နဲ့						
<u>i</u>				S	N>=5 Solid	0	E 2	ر	- 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1						End of borehole @ 15.57m bgl (target depth).
					Bouncir		Ē	16	-						
							2	2	-						
COMMENTS:															



BOREHOLE No.: BH109

CO-ORDINATES: (NZTM2000) R.L.: DATUM:	5179 ² 15614 38.31 CCD	456.0							DRIL	L TYPE L MET L FLUI	HOD:	SNC			HOLE STARTED: 28/09/2018 HOLE FINISHED: 28/09/2018 DRILLED BY: ProDrill LOGGED BY: KPS CHECKED: HJB
BEOLOGICAL													E١	IGINE	ERING DESCRIPTION
SEOLOGICAL UNIT, SERIERIC NAME, ORIGIN, MATERIAL COMPOSITION.	28 50 E LUD LOSA (#1)	76 - LOL COOL (77) WATER	CORE RECOVERY (%)	METHOD	TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 50 100 200 (KPa) 200	5 50 20 50 50 50 50 50 50 50 50 50 50 50 50 50	20 60 200 200 200 200 (cm)	Description and Additional Observations
			100 100	SPT PQ HFS	5/4 3/4		38	1 -		W	S				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded. 0.6m - orange and black mottles. 0.8m - wet.
			100	PQ HFS	3/1 N=11		36	2		M-W					Transition material: layered organic silty fine to medium SAND with minor gravel, and organic sandy SILT; dark grey. Wet; sharp organic odour.
			0 100	IFS SPT	2/4 3/9 38 for 70mm		- 35	3 -							Fill: organic and/or granular soils mixed with refuse. Moist to wet.
FILL			100 100	SPT PQ HFS	N>=50 Bouncing 2/4 4/2		34	4 -							For a general description of the landfill materials see the Geotechnical
			100	PQ HFS	2/4 N=12		33	5 -							Assessment Report. Detailed field observations of the landfill material are available on request.
			100	S SPT	1/0 1/1 1/1 N=4		32	6 -							
			100 100	SPT PQ HFS	0/0		31	7 -							
			100 1	PQ HFS S	1/0 1/1 N=3		∎ 	8 -							
		28/09/2018 10.3 m hdl	100	SPT	3/15 20/15 15 for 75mm		29	9 -		S	VD				Sandy fine to coarse GRAVEL with minor silt and amorphous organics; dark grey. Very dense, wet to saturated, well graded. Gravel, subangular to subrounded; sand, fine to coarse. 9.8m - organics absent; greyish brown.
		A 28/	100 100	SPT PQ HFS	N>=50 Bouncing 7/10 15/18		28	10		w					10.6m - grey; wet.
			100	PQ HFS	17 for 5mm N>=50 Solid Bouncing		27	11 -							10.6 to 10.91m - no recovery from SPT; sample obtained from overcore.
NATURAL			1164	SPT	6/12 15/22 13 for 10mm		26	12							12.2 to 12.51m - no recovery from SPT; sample obtained from overcore. 12.3m - greyish brown.
			100	r PQ HFS	N>=50 Solid Bouncing 4/4 6/9		25	13 -			D				
			71 100	AHFS SPT	13/18 N=46 Solid		24	14 -		S					13.7m - trace silt; reddish brown. Saturated, dense. 14.8m - minor silt.
			0	SPT PQ	5/9 13/17 20 for 65mm N>=50	_	23	15	X						14.9 to 15.2m - no recovery. 15.2m - very dense. 15.2 to 15.57m - no recovery from SPT.
OMMENTS:					Solid Bouncing		22	16 -							End of borehole @ 15.51m bgl (target depth).



BOREHOLE No.: BH110

CO-ORDINATES: (NZTM2000)	5179 1561												E: MS HOD:				HOLE STARTED: 01/10/2018 HOLE FINISHED: 01/10/2018
R.L.:	38.22																DRILLED BY: ProDrill
DATUM: GEOLOGICAL	CCD											- FLUI	D: WA	IER	EI		LOGGED BY: KPS CHECKED: HJB ERING DESCRIPTION
GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.	13	50 FLUID LOSS (%)	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SMAPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 SHEAR STRENGTH 100 (kPa) 200	1 5 20 51 100 50 60 60 60 60 60 60 60 60 60 60 60 60 60	20 60 200 DEFECT SPACNG 200 (cm) 200	Description and Additional Observations
			-	100	PQ HFS				38			M	S				Capping material: SILT with some sand, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlet organic odour; sand, fine to medium.
									37	1	\otimes	W					0.4m - minor gravel; dry to moist, "firm to stiff". Grav fine to coarse, subangular to subrounded.
			-	100 100	PQ HFS SPT		2/5 9/7 7/6 N=29		36	2							Transition material: SILT with some sand, minor gravel, amorphous organics; brown to dark brown. "Firm to stiff", moist, low plasticity, very slow dilatan Contains trace brick and timber; organic odour; san fine to medium.
			-	100	SPT PQ		2/1 1/1		- 35	3							1.0m - interbedded silty fine to medium SAND with minor gravel, fine to medium SAND, and organic sandy SILT. Wet.
				100	PQ HFS		2/2 N=6		34	4							Fill: organic and/or granular soils mixed with refuse. Moist to wet.
FILL				0	SPT		2/1 0/1 2/3			5	\boxtimes						4.5 to 5.0m - no recovery from SPT; sample not recovered from overcore.
				100	PQ HFS		N=6		33	6	\bigotimes						For a general description of the landfill materials see the Geotechnical
-			-	0 100	IFS SPT		1/7 4/3 4/3 N=14		32	7							Assessment Report. Detailed field observations of the landfill material are available on request.
				100	F PQ HFS		4/2		31	1							
- 10° 0 0° 0° X00			-	100	S SPT		3/3 3/4 N=13		- 30	8	\bigotimes						7.6 to 8.05m - no recovery in SPT.
â 				100	PQ HFS					9 -	\mathbb{X}						8.55 to 9.1m - no recovery.
				100	SPT		3/5 4/5 7/19		- 29 -		0.000 2.000	S	D				Sandy fine to coarse GRAVEL with minor cobbles a trace silt; bluish grey. Dense, saturated, well grader Gravel, subangular to subrounded; sand, fine to
		0 000	m bgl	100	PQ HFS		N=50		28	10							coarse. 9.4 to 9.6m - wood pieces.
			11.1 m bgl	100 -	PQ HFS		35/15 or 60mm N>=50 Solid Bouncing		27	11			VD				9.6m - minor cobbles, trace silt. 10.6m - very dense. 10.6 to 10.74m - no recovery from SPT; sample obtained from overcore. 12.0m - minor silt, trace cobbles; greyish brown.
NATURAL				100	SPT		20/37 50		26	12							12.2 to 12.41m - no recovery from SPT; sample
11. °C 11. °C 10.				100	PQ HFS		or 55mm N>=50 Solid Bouncing		25	13	0.0 0.0 0.0 0.0 0.0						obtained from overcore. 12.6m - brown. 13.1m - orange-brown.
			-	100	SPT		15/25 20/22 8 or 40mm			14	0.0 0.8 0.0						13.7 to 14.04m - no recovery from SPT; sample obtained from overcore.
				100	PQ HFS		N>=50 Solid Bouncing 11/17		24	15							
				0	SPT		24/26 or 65mm N>=50		23		×						15.2 to 15.49m - no recovery from SPT. End of borehole @ 15.49m bgl (target depth).
							Solid Bouncing		22	16							of selections of the term of (target deput).
COMMENTS:	!								1		1						



BOREHOLE No.: BH111

PROJECT: Kyle														, Wate	rloo Re	oad, Hornby JOB No.: 1003207.0000
CO-ORDINATES: (NZTM2000)	5179 1561										L TYPI					HOLE STARTED: 04/10/2018 HOLE FINISHED: 04/10/2018
R.L.:	38.61	1m								DRIL	L MET	HOD:	SNC			DRILLED BY: ProDrill
DATUM:	CCD									DRIL	L FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL														E	NGINE	EERING DESCRIPTION
GEOLOGICAL UNIT, GENERIC NAME,																
ORIGIN, MATERIAL COMPOSITION.											MEATHERING		ENGTH	SNE	DEFECT SPACING (cm)	Description and
	100	SS (%)		RECOVERY (%)		TESTS						ENSITY ION	SHEAR STRENGTH (KPa)	DOMPRESSIVI STRENGTH (MPa)	DEFECT	Additional Observations
		FLUID LOSS (%)	¥.	E RECO	D D		SAMPLES	Ê	DEPTH (m)	GRAPHIC LOG	MOISTURE	STRENGTH/DENSIT CLASSIFICATION	SHE	8.,		
	81	82	WAIER	CORE RE	CASING		SAME	RL (m)	DEPT	GRAF			866829 866829	20050-	88888	
								Ē		\otimes	м	s				Capping material: SILT with some sand, amorphous organics; brown to dark brown. "Soft", moist, low
				86 PO HES				38		\otimes						plasticity, very slow dilatancy. Contains trace rootlets organic odour; sand, fine to medium.
					-			Ē	1 -	\otimes	M-W					0.35m - trace gravel, fine to medium, subangular to
					_	1/1		37		\mathbb{X}						subrounded. Fill: organic and/or granular soils mixed with refuse.
			1	100 Tax	5	0/1 2/3		∎_ 0/	2 -	\otimes						1.0m - moist to wet.
5						N=6		Ē	2	\otimes						1.3 to 1.5m - no recovery.
				100 PO HES	3			36		\otimes						
			┝	+	\dashv			Ē	3 -	\otimes						No SPT @ 3.0m (core slipped out of barrel).
				y.				E ar		\otimes						
				83 PO HES	3			35		\otimes						
FILL					-			Ē	4 -	\bigotimes						4.25 to 4.5m - no recovery.
				700 SPT		1/2	-	34		∞						4.25 to 4.5m - no recovery.
=			-			1/2 3/5		∎ F	5	\otimes						For a general description of the longitil
5				PO HES		N=11		Ē		\otimes						For a general description of the landfill materials see the Geotechnical
				- G	ž			33		\otimes						Assessment Report.
				200 201	-	2/2	-	Ē	6 -	\otimes						Detailed field observations of the landfill material are available on request.
			-			2/6 5/3		32		\otimes						
				PO HES		N=16		Ē	7 -	\otimes						
					-			Ē		\otimes	W-S					7.4m - wet to saturated.
				100 S PT		4/3 4/6		31		\otimes						7.4m - Wel to Saturateu.
5						4/5 N=19			8 -	\otimes						
; ;				PO HES				30		88	1	VD				Sandy fine to coarse GRAVEL with trace to minor silt
<u></u>								Ē	9 -							and trace cobbles; yellowish brown. Very dense, wet to saturated, well graded. Gravel, subangular to
		8		5 10		11/20 50	F	È			1					subrounded; sand, fine to coarse.
		9/201	n bg			for 65mm N>=50		29								
I.		20/09/2018	10.4	PO HES	3	Bouncing		Ē	10	000						
			L			13/27		28			1					10 Cm
f Solo			╞	6 F		42/8 for 2mm		Ē	11 -							10.6m - greyish brown. 10.6 to 10.83m - no recovery from SPT; sample
				100 HES		N>=50 Solid		Ē								obtained from overcore.
				PO HES	3 -	Bouncing		- 27 E		$\hat{\mathbf{p}}_{\mathbf{p}}$						
NATURAL						7/7		Ē	12 -		1					
				100 SPT	5	8/12 15/15		26								12.2 to 12.65m - no recovery from SPT; sample obtained from overcore.
						N>=50 Solid		Ē	13 -							
				PO HES	3			Ę	-	\hat{o}, \hat{o}						
<u> </u>			┝			6/6 8/5		25		R°,	1					13.7m - saturated, medium dense.
			-		5	3/3 N=19		Ē	14	\downarrow	L					13.7 to 14.15m - no recovery from SPT; sample not
				95 PO HES		Solid		24			s	MD				obtained.
				<u>م</u> ا	<u>×</u>	5/4 5/7		Ē	15 -							
					-	7/6 N=25		Ę	-	\sim	1					15.2 to 15.65m - no recovery from SPT; sample
			+		<u>י</u>	Solid		23								obtained from overcore.
								Ē	16 -	-						End of borehole @ 15.65m bgl (target depth).
COMMENTS:								F		-						



BOREHOLE No.: BH112

PROJECT: Kyle	517	017	3 00) m^	J						יווסח	TVD	N: Kyle E: MS	1000			HOLE STARTED: 04/10/2018
(NZTM2000)	156																HOLE FINISHED: 04/10/2018
R.L.:	38.4												HOD:				DRILLED BY: ProDrill
	CCI	2									DRILI	L FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB
		Г								_					El		ERING DESCRIPTION
GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		25 50 FLUID LOSS (%) 75	WATER	CORE RECOVERY (%)	метнор	CASING	TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 50 SHEAR STRENGTH 100 (kPa) 200	1 5 20 STRENGTH 50 (MPa) 250 (MPa)	20 60 200 DEFECT SPACING 200 (am) 200	Description and Additional Observations
			>	0	~	0		s	2	۵	Ŵ	М	s				Capping material: SILT with some sand and trace
		· · · · · ·		100	PQ HFS				38	1 -		D-M	L				gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
			ľ	100	SPT		1/3 3/3		37		\bigotimes						Transition material: sandy fine to coarse GRAVEL v trace silt; brown. "Loose", dry, well graded. Gravel,
			-	100	PQ HFS		2/2 N=10		36	2 -							angular to subrounded; sand, fine to coarse. 0.6m - minor to some silt, trace wood fibres; moist. 0.95 to 1.05m - orange mottles. 1.5m - organic; dark brown to black. Organics
				53	PQ HFS				35	3 -							\amorphous and fibrous. Fill: organic and/or granular soils mixed with refuse No SPT @ 3.0m (wood).
FILL					PQ					4	\mathbf{N}						3.8 to 4.5m - no recovery.
			ł	100	SPT		1/1 1/1		34		\bigotimes						
				100	PQ HFS		1/1 N=4		33	5 -							For a general description of the landfill materials see the Geotechnical Assessment Report.
			ł	100	SPT		3/4 2/2		- 32	6	\otimes						Detailed field observations of the landfill material are available on request.
			ļ	100	PQ HFS		2/2 N=8		- 32	7 -							
				100	SPT		4/6 8/7		31		*						
				61	PQ HFS		6/5 N=26		30	8 -		W-S	VD				Sandy fine to coarse GRAVEL with minor cobbles a trace silt, amorphous organics; dark brownish grey.
			-	1	141		7/43 for 65mm N>=50	_	29	9 -	0.0.0	S					Very dense, wet to saturated, well graded. Gravel, subangular to subrounded; sand, fine to coarse. 8.6m - organics absent.
			10.8 m bgl	100	PQ HFS		Bouncing		- 28	10 -							 8.7 to 9.1m - no recovery. 9.2m - silty; golden brown staining, petrol odour. Si plastic.
			V	100	SPT		18/18 25/25 for 75mm		20	11 -							9.3m - trace silt. 9.6 to 9.8m - bluish grey; saturated. Strong petrol
NATURAL				100	PQ HFS		N>=50 Solid Bouncing		27		0.0.0.0 0.0.0 0.0.0						odour. 10.5m - bluish grey. 10.6 to 10.9m - no recovery from SPT; sample obtained from overcore. 10.8 to 11.2m - silty; golden brown staining, petrol
				100	SPT		7/10 18/12 14/6		26	12 -							odour. Silt is plastic. 11.0m - trace silt; bluish grey.
				100	PQ HFS		for 25mm N>=50 Solid Bouncing			13 -	0.000						12.2 to 12.6m - no recovery from SPT; sample obtained from overcore. 12.7m - brown.
				100	SPT P		10/17 15/10		25		0.00 0.00 0.00						13.7 to 14.08m - no recovery from SPT; sample
				100 1	PQ HFS s		20/5 for 5mm N>=50 Solid Bouncing		24	14 -	0.000						obtained from overcore.
				_	A	+	_ our rolling		23	15							SPT not recorded @ 15.2m. End of borehole @ 15.2m bgl (target depth).
										16 -							Line of potoniolo (gr 10.2111 byr (talyet ueptir).
OMMENTS:								1	- 22				I				



BOREHOLE No.: BH113

PROJECT: Kyle CO-ORDINATES:	517909	90 00) mN								-				Dad, Hornby JOB No.: 1003207.0000 HOLE STARTED: 02/10/2018
(NZTM2000)	156138														HOLE FINISHED: 02/10/2018
R.L.:	38.12m	ı								L MET					DRILLED BY: ProDrill
DATUM:	CCD								DRIL	L FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL													EI		ERING DESCRIPTION
GEOLOGICAL UNIT, GENERIC NAME,														DN N	
ORIGIN, MATERIAL COMPOSITION.			(%)							THERING	>	SHEAR STRENGTH (KPa)	SSIVE GTH (a)	DEFECT SPACING (cm)	Description and Additional Observations
	(%) SSO		OVERY (TESTS				90	, WEA	ATION	IEAR ST (kP.	COMPRESSIV STRENGTH (MPa)	DEFEC	
	28 50 FLUID LOSS (%) 75	WATER	CORE RECOVERY (%)	CASING		SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION				
	882	Ŵ	8 4	1 3		S.	ੇ - 	В	×××	¥8 M	S S	82822 82822	5 5 5 5 100 250	2000 20 20 20 2000 20 20 2000 20 20	Capping material: SILT with some sand and trace
			u u	, l				-	\otimes						gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy.
			100 PO HES	3				4 -	XX		F-St				Contains trace rootlets; organic odour; sand, fine to
							37	1	\otimes						medium; gravel, fine to medium, subangular to subrounded.
Ē			100 Tax	-	1/1 2/2			-	\otimes						0.2m - sandy; brown mottled yellowish brown. Transition material: Gravelly sandy SILT; brown. Mo
X 200					5/4 N=13		- 36	2 -	\bigotimes						low plasticity. Contains trace brick; gravel, fine to
× n			100 PO HES	Í			_		\otimes						coarse, subangular to subrounded; sand, fine to coarse.
								3 -	\otimes						0.8m - gravel absent; dark brown. "Firm to stiff". 1.2 to 1.4m - trace sand; grey mottled orange and d
		[100 SPT	5	1/1 1/1		- 35	3	\otimes						brown. Moderate plasticity, no dilatancy. 1.4m - trace gravel, medium to coarse, subangular t
FILL		[1/1 N=4		-	-	\otimes						subrounded.
			100 PO HES	3			- 34	4 -	\otimes						Fill: organic and/or granular soils mixed with refuse.
			[_]	-					\otimes						
								5 -	\otimes						No SPT @ 4.5m (wood).
			50 PO HES				- 33	5	R7						5.1 to 6.1m - no recovery (timber blocked barrel).
_								-	X						
5 6 7					2/1		- 32	6 -	\sim	s					
			100 100	5	3/4 4/4				\bigotimes						6.1m - saturated.
E007			61 PO HES	2	N=15			7 -	\sim	w					6.55 to 6.95m - no recovery.
			61 PO H	Ž			- 31		\otimes	vv					6.95m - wet.
ă			-	+					\sim						End of borehole @7.6m bgl (refusal on steel).
							- 30	8 -	-						
							_								For a general description of the landfill
								9 -							materials see the Geotechnical
															Assessment Report. Detailed field observations of the landfill
							-	-	-						material are available on request.
							- 28	10 -							
							_								
							c=	11 -							
							-	-							
								12 -							
								-							
								13 -							
							- 25	-							
							- 24	14 -							
							-								
							- 23	15 -							
						IE		-							
							-								
							- 22	16 -							
COMMENTS:	111					F			1					mii	
lole Depth															



BOREHOLE No.: BH114

PROJECT: Kyle CO-ORDINATES: (NZTM2000)	51790 15613								DRIL	L TYPI	E: MS	1000			Dad, Hornby JOB No.: 1003207.0000 HOLE STARTED: 03/10/2018 HOLE FINISHED: 03/10/2018
R.L.:	39.00n	n							DRIL	L MET	HOD:	SNC			DRILLED BY: ProDrill
DATUM:	CCD								DRIL	L FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB
													EN	IGINE	
GERCEIDLAL UNIT, GERERIC NAME, ORIGIN, MATERIAL COMPOSITION.	SS FLUID LOSS (%)	WATER	CORE RECOVERY (%)	METHOD	TESTS		Sterrics RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE MEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 50 100 200 (MPa) 200	1 5 20 20 87RENSTH 100 (MPa) 280	20 60 DEFECT SPACING 200 (cm) 2000	Description and Additional Observations
			86	PQ HFS			- 38	1		M-W	S				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to
			100	SPT	3/3 3/2										subrounded. Transition material: gravelly SILT with some sand; brown. "Soft", moist, low plasticity, no dilatancy.
			100	PQ HFS	2/2 N=9		37	2							Contains trace timber, metal, brick, plastic, white paint/plaster chips.
					5/8		36	3							0.9 to 1.1m - no recovery. Fill: organic and/or granular soils mixed with refuse
			100	FS SPT	5/4 5/3 N=1										
			100	PQ HFS			35	4							For a general description of the landfill materials see the Geotechnical Assessment Report.
FILL			100	SPT SPT	32/3 3/4 4/2 N=1	-	34	5							Detailed field observations of the landfill material are available on request.
			100	PQ HFS											
			22	SPT	4/1 25/1 5/8	2	- 33	6	\bigotimes						6.1 to 6.55m - no recovery from SPT; 100mm obtai from overcore.
			80	PQ HFS	N>= Bound		32	7							
			100	SPT	3/4 4/4 5/5		31	8							7.4 to 7.6m - no recovery.
			100	PQ HFS	N=1	8	30	9							
		018 ogl	100	PQ HFS	10/4 for 75 N>=5 Bound	mm 50	29	10							 9.1 to 9.25m - no recovery from SPT; sample not obtained. 9.1 to 10.6m - drilling equipment damaged; retrieva equipment lost downhole may have resulted in mix core.
		 03/10/2018 11.1 m bgl 	100	SPT	12/1 14/2 10	6	28	11		W-S	VD				Sandy fine to coarse GRAVEL with trace silt; greyis brown. Very dense, wet to saturated, well graded.
			100	PQ HFS	for 30 N>= Soli Bound	mm 5 0 d									Gravel, subangular to subrounded; sand, fine to coarse. 10.6 to 10.93m - no recovery from SPT; sample obtained from overcore.
			100	SPT	9/10 10/1 11/1 N=4	0	- 27	12	0 (00 000		D				12.2m - dense. 12.2 to 12.65m - no recovery from SPT; sample
NATURAL			100	PQ HFS	N=4 Soli		26	13	0.00						obtained from overcore.
			100	SPT	7/5 4/5 5/4 N=1		25	14	0.0 0.0 0.0		MD				13.7m - medium dense. 13.7 to 14.15m - no recovery from SPT; sample
			100	PQ HFS	Soli 5/1:	2									obtained from overcore.
			0	SPT	19/ 8/9 N=4 Soli	1	24	15							15.2m - dense. 15.2 to 15.65m - no recovery from SPT.
							23	16							End of borehole @ 15.65m bgl (target depth).



BOREHOLE No.: BH115

CO-ORDINATES: (NZTM2000)	Park 51792 15615	25.0								L TYPE					HOLE STARTED: 05/10/2018 HOLE FINISHED: 05/10/2018
R.L.: DATUM:	38.66r CCD	m								L FLUI					DRILLED BY: ProDrill LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL											D. 117		EN	NGINE	ERING DESCRIPTION
GEOLOGICAL UNIT, GENERIC NAME. ORIGIN, MATERIAL COMPOSITION.	20 FLUID LOSS (%)	75 V. WATER	CORE RECOVERY (%)	метнор	TESTS	SMIPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (KPa)	COMPRESSIVE STRENGTH (MPa)	DEFECT SPACING (cm)	Description and Additional Observations
FILL	83	52 · · · · · · · · · · · · · · · · · · ·	60	PQ HFS ME	8	8	- 38	1	20 20	M	S	10 25 100	1 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6		Capping material: SILT with some sand, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets organic odour; sand, fine to medium. 0.3m - trace gravel, fine to medium, subangular to subrounded. Transition material: SILT with some sand and trace
			76	PQ HFS			37	2 -		M-W W	MD				gravel, amorphous organics; dark brown mottled ligh brown and orange. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odou sand, fine to medium; gravel, medium to coarse, subangular to subrounded. 0.75m - some organics, spongy, brown.
		-	100	SPT	4/3 5/5		E	3 -	0,00						Fill: organic and/or granular soils mixed with refuse.
			100	PQ HFS	4/4 N=18		35	4 -	0.0.0						0.9 to 1.5m - no recovery. No SPT @ 1.5m (wood). 2.25 to 2.6m - no recovery.
			100	SPT	6/17 12/8 12/11		- 34	5 -	0.0.0.0 0.0.0.0		D				Sandy fine to coarse GRAVEL with trace to minor si and trace cobbles; grey. Medium dense, wet, well graded. Gravel, subangular to subrounded; sand, fir
			100	PQ HFS	N=43 Solid		33		00000						to coarse. 4.5m - dense. 4.5 to 4.95m - no recovery from SPT; sample obtain from overcore.
			100	SPT	14/14 14/16 20		- 32	6 -			VD				6.1m - very dense. 6.1 to 6.48m - no recovery from SPT; sample obtain from overcore.
			100 100	SPT PQ HFS	for 75mm N>=50 Solid Bouncing 12/14 18/18		- 31	7 -	0.0.0.0.0						7.6 to 7.97m - no recovery from SPT; sample obtain
		-	46	PQ HFS	14 for 70mm N>=50 Solid		30	8 -							from overcore. 8.5 to 9.1m - no recovery.
NATURAL			100 100	SPT	6/16 35/15 for 15mm N>=50		29	9 -	0.0.00						9.1m - greyish brown. 9.1 to 9.34m - no recovery from SPT; sample obtain from overcore.
		05/10/2018 11.1 m bgl		- PQ HFS	Solid Bouncing 8/8		28	10 -	0.0 0.0 0.0						
		▲ 11	_	FS SPT	10/10 11/13 N=44 Solid			11 -	0,		D				10.6m - dense. 10.6 to 11.05m - no recovery from SPT; sample obtained from overcore.
-			100	r PQ HFS	8/16 11/8		27	12 -	0.00	S					11.9m - brown.
			100	HFS SPT	8/10 N=37 Solid		26	13 -		5					12.2m - saturated. 12.2 to 12.65m - no recovery from SPT; sample obtained from overcore.
			0 61	PQ	5/6 5/8		25								13.3 to 13.7m - no recovery.
			0 100	IFS SPT	10/12 N=35 Solid			14 -	0.000						13.7 to 14.15m - no recovery from SPT; sample obtained from overcore.
			100	SPT PQ HFS	6/6 5/7 9/13		24	15 -	0:0 0:0 0:0						15.2 to 15.65m - no recovery from SPT.
			0	Я	N=34 Solid		- 23	16 -							End of borehole @ 15.65m bgl (target depth).



BOREHOLE No.: BH116

PROJECT: Kyle CO-ORDINATES: (NZTM2000) R.L.:	51791 15615 36.20r	54.0								DRIL DRIL	L TYPI L MET	E: MS HOD:	1000 SNC			Add, Hornby JOB No.: 1003207.0000 HOLE STARTED: 05/10/2018 HOLE FINISHED: 05/10/2018 DRILLED BY: ProDrill
DATUM: GEOLOGICAL	CCD									DRIL	L FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB ERING DESCRIPTION
									-	-						
GENERIC MAME, ORIGIN, MATERIAL COMPOSITION.	80 FLUID LOSS (%)	WATER	CORE RECOVERY (%)	метнор	CASING	TESTS	SMAPLES	KL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 50 100 200 (kPa) 200	1 5 20 20 50 817EENGTH 100 (MPa) 250 250	20 60 DEFECT SPACING 200 000 (cm) 2000	Description and Additional Observations
			76	PQ HFS				36	1		М	S				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
			100	SPT		1/1 0/1					W-S					Transition material: SILT with some sand and minor gravel, amorphous organics; brown to dark brown.
			0	PQ HFS		2/1 N=4		34	2							"Soft", moist, low plasticity, very slow dilatancy. Contains trace concrete and bark; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
			30	PQ HFS				33	3							0.95 to 1.3m - no recovery. Fill: organic and/or granular soils mixed with refuse. Wet to saturated. 1.95 to 3.0m - no recovery. No SPT @ 3.0m (metal, core loss).
FILL			100	SPT		1/1 2/2			-							3.45 to 4.5m - no recovery.
			100	PQ HFS		4/4 N=12		31	5							For a general description of the landfill materials see the Geotechnical Assessment Report.
5			100	SPT		5/7 5/4 2/2		30	0	\otimes						Detailed field observations of the landfill material are available on request.
			100	PQ HFS		N=13		29	7							
			33	SPT		2/2 1/2 0/1		- 28	8	\otimes						7.6 to 7.9m - no recovery in SPT; 150mm obtained from overcore.
			100	- PQ HFS		N=4			9							
		18	100	SPT		2/0 1/2 12/35										
		05/10/2018 10.6 m bal		r PQ HFS		for 70mm N>=50 Bouncing 7/7		26	10		w s	D				Sandy fine to coarse GRAVEL with minor silt and the cobbles; bluish grey. Dense, wet, well graded. Graves subangular to subrounded; sand, fine to coarse. 10.4m - brownish grey.
			100	SPT		8/10 10/10 N=38		25	11	0.00						10.6m - trace silt; saturated, brown. 10.6 to 11.05m - no recovery from SPT; 50mm
			100	PQ HFS		Solid 9/12		24	12							obtained from overcore.
NATURAL			100	SPT		14/14 16/6 for 15mm		- 24 -				VD				12.2m - very dense. 12.2 to 12.59m - no recovery from SPT; sample obtained from overcore.
			100	PQ HFS		N>=50 Solid Bouncing 7/10		23	13							
			100	SPT		16/16 16/2 for 5mm		22	14							13.7 to 14.08m - no recovery from SPT; sample obtained from overcore.
			100	PQ HFS		N>=50 Solid Bouncing 19/20			15							
			0	SPT		25/25 for 75mm N>=50		21		\geq	1					15.2 to 15.5m - no recovery from SPT.
						N>=50 Solid Bouncing		20	16							End of borehole @ 15.50m bgl (target depth).
COMMENTS: Hole Depth 15.5m								20	10							



BOREHOLE No.: BH117

CO-ORDINATES: (NZTM2000)	5179 1561												E: Fra		1		HOLE STARTED: 06/10/2018 HOLE FINISHED: 06/10/2018
R.L.:	36.1												HOD:				DRILLED BY: ProDrill
DATUM: GEOLOGICAL	CCE)									DRILL	. FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB ERING DESCRIPTION
GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		80 FLUID LOSS (%)	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SMAPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 26 SHEAR STRENGTH 50 (kPa) 200	1 5 20 20 50 60 (MPa) 50 50 50 50 50 50 50 50 50 50 50 50 50	20 60 200 200 200 200 (cm)	Description and Additional Observations
			-	83	PQ HFS				36	1 -		M	S				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
				0	SPT		0/0 0/0				$\overset{\infty}{\searrow}$						Transition material: gravelly SILT with some sand, amorphous organics; brown to dark brown. "Soft",
				80	PQ HFS		0/1 N=1		34	2 -	\bigotimes	M-W					moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded.
			-	22	SPT		0/1 1/0		33	3 -	\bigotimes						0.75 to 1.0m - no recovery. 1.5 to 2.15m - no recovery.
			ļ	100	PQ HFS		1/5 N=7			4 -	\bigotimes						Fill: organic and/or granular soils mixed with refuse Moist to wet. 3.1 to 3.45m - no recovery.
FILL			-	44	SPT PC		4/2 2/1		32		\bigotimes						
				69	PQ HFS		1/2 N=6		31	5 -	\bigotimes						4.7 to 5.3m - no recovery. For a general description of the landfill
			-	100	SPT PQ		1/1		- 30	6 -	\bigotimes						materials see the Geotechnical Assessment Report.
			-	100	PQ HFS SI		1/1 1/2 N=5		-	7 -	\bigotimes						Detailed field observations of the landfill material are available on request.
			-	100	SPT PQ		3/4		29		\bigotimes						
			-	76 10	PQ HFS SI		3/3 2/2 N=10		28	8 -							8.05 to 8.3m - no recovery.
			-	100 7	spt PQ		8/10		- 27	9 -		W	VD				Sandy fine to coarse GRAVEL with trace to minor cobbles and silt; dark grey. Very dense, wet, well graded. Gravel, subangular to subrounded; sand, fi to coarse.
		c	•_	100	PQ HFS		14/21 15 for 50mm N>=50			10 -							8.8m - grey.
			11.0 m bgl	100	SPT PG		Bouncing 4/4 4/4		26			S	MD				10.6m - medium dense, saturated.
		-	▼_	100			3/5 N=16		25	11 -							11.0m - brown.
NATURAL					T PQ HFS		5/5 4/4		24	12 -							
		ļ		0 100	HFS SPT		6/7 N=21			13 -							
			-	0 100	T PQ HFS		6/6 7/7		23		0.0.0 0.0.0 0.0.0						
			-	100	HFS SPT		4/5 N=23		22	14 -							
			-	0 100	SPT PQ HFS		3/4 4/5 5/7		- 21	15 -	0.00 0.00 0.00 0.00						
				5	Ŗ		N=21			16 -	127						End of borehole @ 15.65m bgl (target depth).
COMMENTS:									20		1						



BOREHOLE No.: BH118

CO-ORDINATES: (NZTM2000)	5179 1561												E: Fras		1		HOLE STARTED: 06/10/2018 HOLE FINISHED: 06/10/2018
R.L.:	36.2												HOD:				DRILLED BY: ProDrill
DATUM: GEOLOGICAL	CCD)										_ FLUI	D: WA	TER			LOGGED BY: KPS CHECKED: HJB EERING DESCRIPTION
											+						
GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.	*	50 FLUID LOSS (%)	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 50 SHEAR STRENGTH 100 (kPa) 200	1 5 20 50 60 60 60 60 60 60 60 60 60 60 60 60 60	20 60 200 DEFECT SPACING 200 (cm) 200	Description and Additional Observations
			>	66	PQ HFS	0		0	36			M	s				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace rootlets; organic odour; sand, fine to
							0.40		35	1 -	\bigotimes						medium; gravel, fine to medium, subangular to subrounded. Transition material: gravelly SILT with some sand,
			-	1 100	HFS SPT		0/0 1/1 0/1 N=3		34	2 -	\bigotimes						amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy. Contains trace clinker; organic odour; sand, fine to medium;
			-	100 61	SPT PQ HFS		0/0		- 33	3 -	\bigotimes						gravel, fine to medium, subangular to subrounded. 0.9m - contains trace brick. 1.0 to 1.5m - no recovery.
				100 1	PQ HFS SI		0/0 2/2 N=4			4 -	\bigotimes						\2.0 to 2.4m - no recovery. Fill: organic and/or granular soils mixed with refuse.
				100	SPT PG		1/1 0/1		32		\bigotimes						For a general description of the landfill materials see the Geotechnical Assessment Report.
FILL				82	PQ HFS		0/0 N=1		31	5 -							Detailed field observations of the landfill material are available on request.
			-	100	SPT		1/1 1/0		30	6	\bigotimes						5.9 to 6.1m - no recovery.
-				100	PQ HFS		0/0 N=1		29	7 -	\bigotimes						
			-	100	SPT		2/2 1/2 3/4		- 28	8 -							
-				100	PQ HFS		N=10			9 -	\bigotimes						
				100	SPT SPT		2/3 3/2 3/3 N=11		27		\bigotimes						
			▲ 10.8 m bgl	100	r PQ HFS		14/12		26	10 -		w S	. D				Sandy fine to coarse GRAVEL with trace to minor s and trace cobbles; bluish grey. Dense, wet, well
-			•	100	FS SPT		10/10 10/12 N=42		25	11 -	0.00						graded. Gravel, subangular to subrounded; sand, fi to coarse. 10.3m - saturated.
				100	PQ HFS		7/10		24	12 -							11.0m - brown. 11.8m - minor sand, trace silt.
NATURAL			-	100	SPT		12/13 20/5 for 10mm N>=50			10			VD				12.2m - very dense.
			-	100	r PQ HFS		Bouncing 7/7 7/7		23	13 -			D				40.7m dana
-				100 100	PQ HFS SPT		8/8 N=30		22	14 -							13.7m - dense.
			-	0	SPT PQ F		6/6 8/9 8/10 N=35		21	15 -							
					0				20	16 -							End of borehole @ 15.65m bgl (target depth).
COMMENTS:									-		1					min	



BOREHOLE No.: BH119

CO-ORDINATES:	Park 517918								DRILI	L TYPI	E: MS	1000			HOLE STARTED: 06/10/2018
(NZTM2000)	156164) mE	Ξ					DRILI	L MET	HOD:	SNC			
R.L.: DATUM:	35.85m CCD	n							DRILI	L FLUI	D: WA	TER			DRILLED BY: ProDrill LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL													El	NGINE	ERING DESCRIPTION
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.	88 FLUID LOSS (%)	WATER	CORE RECOVERY (%)	метнор	TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING	STRENGTH/DENSITY CLASSIFICATION	10 25 50 SHEAR STRENGTH 100 (kPa) 200	1 5 20 50 50 50 50 50 50 50 50 50 50 50 50 50	20 60 200 DEFECT SPACING 200 (cm) 2000	Description and Additional Observations
	NIDE		100 0	PQ HFS		0				M	S				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown. "Soft", moist, low plasticity, very slow dilatancy.
					3/2		35	1 -	\bigotimes						Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to subrounded. Transition material: SILT with some sand and mino
			100 100	PQ HFS SPT	1/0 1/1 N=3		- 34 	2 -							gravel, amorphous organics; brown to dark brown mottled yellowish brown. "Soft", moist, low plasticit; very slow dilatancy. Contains trace brick; organic odour; sand, fine to medium; gravel, fine to medium
-			100	SPT PC	2/11 8/6 2/2		33	3 -							subangular to subrounded. Fill: organic and/or granular soils mixed with refuse
			99	PQ HFS	N=18		32	4 -	\bigotimes						3.45 to 3.8m - no recovery.
FILL			0	S SPT	1/1 0/1 1/2 N=4		31	5 -	\bigotimes						4.5 to 5.0m - no recovery.
			0 100	T PQ HFS	1/1		30	6 -							For a general description of the landfill materials see the Geotechnical
-			100 100	PQ HFS SPT	2/2 3/8 N=15		29	7 -	\bigotimes						Assessment Report. Detailed field observations of the landfil material are available on request.
			100	SPT PG	3/1 2/2 3/3		- 28	8 -	\bigotimes						
			100	PQ HFS	N=10		27	0 -	\bigotimes						
			100	S SPT	5/4 5/7 5/5 N=22		- 26	9							
		06/10/2018 11.3 m bgl	100 100	SPT PQ HFS	18/18			10 -		W	VD				Sandy fine to coarse GRAVEL with trace to minor and trace cobbles; bluish grey. Very dense, wet, w graded. Gravel, subangular to subrounded; sand, f
		▲ ^{06/1}	100	PQ HFS s	18/18 14 for 55mm N>=50 Solid Bouncing		25	11 -	0.000						to coarse. 10.6m - brownish grey; saturated. 10.6 to 10.96m - no recovery from SPT; sample obtained from overcore.
			100	SPT	10/12 24/26 for 35mm N>=50		24	12	0.00.0						11.2m - brown.12.2 to 12.46m - no recovery from SPT; sample obtained from overcore.
NATURAL			100	PQ HFS	Solid Bouncing 5/12		23	13 -	0.000						
			0 100	IFS SPT	12/9 12/15 N=48 Solid		22	14 -	0.0.0 0.0.0						13.7 to 14.15m - no recovery from SPT; sample obtained from overcore.
			0 100	SPT PQ HFS	7/10 15/15 20 for 65mm		21	15							15.2 to 15.57m - no recovery from SPT.
					N>=50 Solid Bouncing		20	16 -	-						End of borehole @ 15.57m bgl (target depth).



BOREHOLE No.: BH120

PROJECT: Kyle CO-ORDINATES:	51792	207	.00	mN								E: MS		.,		oad, Hornby JOB No.: 1003207.0000 HOLE STARTED: 06/10/2018
(NZTM2000)	15616	664										HOD:				HOLE FINISHED: 06/10/2018
R.L.: DATUM:	36.10 CCD	m										D: WA				DRILLED BY: ProDrill LOGGED BY: KPS CHECKED: HJB
GEOLOGICAL												J. 11F		F	NGIN	ERING DESCRIPTION
GEOLOGICAL UNIT,														_		
GENERIC NAME, ORIGIN,											SNIS		GTH	¥-	DEFECT SPACING (cm)	Description and
MATERIAL COMPOSITION.	(36)	101.1		RY (%)		TESTS					WEATHERING	N N	SHEAR STRENGTH (KPa)	COMPRESSIV STRENGTH (MPa)	FECT SF (am)	Description and Additional Observations
	25 50 FLUD LOSS (%)			RECOVERY (%)			ES		Ē	GRAPHIC LOG		STRENGTH/DENSITY CLASSIFICATION	SHEAF	ST	B	
	12	82	WATER	CORE	METHOD		SAMPLES	RL (m)	DEPTH (m)	GRAPH	MOISTURE	STREN	20 20 20 20 20 20 20 20 20 20 20 20 20 2	 50 100	88888	
										\otimes	м	S				Capping material: SILT with some sand and trace gravel, amorphous organics; brown to dark brown.
				93	PQ HFS			Ē		\bigotimes						"Soft", moist, low plasticity, very slow dilatancy.
				0,	g			35	1	\boxtimes						Contains trace rootlets; organic odour; sand, fine to medium; gravel, fine to medium, subangular to
										×						subrounded.
				100	SPT	2/1 2/1		Ē		\otimes						Transition material: SILT with some sand and minor some gravel, amorphous organics; brown to dark
E					HFS	1/1 N=5		34	2	\otimes						brown. "Soft", moist, low plasticity, very slow dilatand Sand, fine to medium; gravel, fine to medium,
Вох 1, 0.0-3.0m				9	DA H			Ē		\otimes						subangular to subrounded.
Box			┝		-			33	3	\bigotimes						Fill: organic and/or granular soils mixed with refuse. 1.4 to 1.5m - no recovery.
					。 			- 33		\bowtie						No SPT @ 3.0m (wood).
				73	PQ HFS			Ē		\otimes						3.0 to 3.4m - no recovery.
					۵			32	4	\otimes						
			H	0	SPT	2/2	_	Ē		2						4.5 to 5.1m - no recovery.
F U 1			H		5	1/0 2/2		- 31	5	\land						
FILL					FIS	N=5				\otimes						For a consul description of the landfill
				86	PQ HFS			Ē		\otimes						For a general description of the landfill materials see the Geotechnical
E				0	-	4/2		30	6	\otimes						Assessment Report.
Box 2, 3,0-6,9m				<u>6</u>	SPT	2/1 2/3		Ē		\otimes						Detailed field observations of the landfill material are available on request.
BX				90	PQ HFS	N=8		Ē	7	\otimes						
				Ę	ğ			- 29		\otimes						
				9	SPT	2/2 1/2	-	Ē		\mathbb{N}						7.6 to 8.05m - no recovery in SPT.
						1/1		28	8	\mathbf{k}	w	-				8.05m - wet.
					HFS	N=5		Ē		\otimes						
					g			Ē	<u>م</u>	\otimes						
				0	SPT	7/6 7/5		- 27 -	0	Ň	1					9.1 to 9.7m - no recovery.
E			F			5/5 N=22		-		\sim						
6.9-10.6m		0/2018	bg	85	PQ HFS			26	10	\otimes						
ö ög		101/90	0.9			14/18		Ē		0.00 0.00	s	VD				Sandy fine to coarse GRAVEL with trace to minor sil
			Č.	100	SPT	15/18		Ē	11	0.00						and trace cobbles; grey. Very dense, saturated, well graded. Gravel, subangular to subrounded; sand, fin
					s	for 70mm N>=50		25		K?/	1					to coarse. 10.6 to 10.97m - no recovery from SPT; sample
				67	PQ HFS	Solid		Ē		\land						obtained from overcore. 11.1m - brown.
						14/16		24	12	0,00						11.2 to 11.8m - no recovery.
				<u>5</u>	SPT	15/15 15/5		E								12.2 to 12.58m - no recovery from SPT; sample obtained from overcore.
S NATURAL					ES	for 5mm N>=50		Ē	13 ·	¢°ċ						
NATURAL				9	PQ HFS	Solid Bouncing		23	13							
Box			$\left \right $			6/14 16/15		Ē								12.7 to 14.09m and account from ODT
				100	SPT	15/4 for 6mm		22	14							13.7 to 14.08m - no recovery from SPT; sample obtained from overcore.
_				100	HFS	N>=50 Solid		Ē								
7-15.6m				¥	PQ HFS	Bouncing 16/16		Ē	15							
Box 5, 13,7-15, 6m				0	SPT	16/18		21	10	\sim						15.2 to 15.57m - no recovery from SPT.
ă			+	-	s	for 70mm N>=50		E		\downarrow						End of borehole at 15.57m bgl (target depth).
						Solid		20	16	1						
						, in the second se		F		-						
COMMENTS:																

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