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Fencing Centre
Qualitative Engineering Evaluation

Reference: 228602
Prepared for:
Christchurch City Council

Functional Location ID: PRO 2394 B003

Address: 95 Jack Hinton Drive, Addington

Revision: 2
Date: 1 July 2013

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Approval			
Author Signature		Approver Signature	
Name	Steven Waldrip	Name	Luis Castillo
Title	Structural Engineer	Title	Senior Structural Engineer

Contents

Appendices

Executive Summary

This is a summary of the Qualitative Engineering Evaluation for the Fencing Centre building and is based on the Detailed Engineering Evaluation Procedure document issued by the Engineering Advisory Group on 19 July 2011, visual inspections, available structural documentation and summary calculations as appropriate.

Building Details	**Name**	Fencing Centre			
Building Location ID	PRO 2394 B003			**Multiple Building Site**	N
Building Address	95 Jack Hinton Drive, Addington			**No. of residential units**	0
Soil Technical Category	TC2	**Importance Level**	2	**Approximate Year Built**	1970
Foot Print (m²)	664	**Storeys above ground**	1	**Storeys below ground**	0
Type of Construction	Portal frame roof, blockwork walls, slab on ground with strip foundation.				

Qualitative L4 Report Results Summary		
Building Occupied	Y	The Fencing Centre is currently in use.
Suitable for Continued Occupancy	Y	The Fencing Centre is suitable for continued occupation.
Key Damage Summary	Y	Refer to summary of building damage Section 3.1 report body.
Critical Structural Weaknesses (CSW)	N	No critical structural weaknesses were identified.
Levels Survey Results	Y	Variations in floor levels were not within the DBH's Guidelines, with falls greater than 1:200 or 0.5%
Building %NBS From Analysis	>100%	Based on an analysis of bracing capacity and demand.

Qualitative L4 Report Recommendations		
Geotechnical Survey Required	Y	Geotechnical survey is required due to damage to the slab.
Proceed to L5 Quantitative DEE	Y	An intrusive investigation of a portion of the slab is required if it is to be repaired to identify repair method.

Approval			
Author Signature	S Waldrip	**Approver Signature**	
Name	Steven Waldrip	**Name**	Luis Castillo
Title	Structural Engineer	**Title**	Senior Structural Engineer

1 Introduction

1.1 General

On 2 March 2012 Aurecon engineers visited the Fencing Centre to carry out a qualitative building damage assessment on behalf of Christchurch City Council. Detailed visual inspections were carried out to assess the damage caused by the earthquakes on 4 September 2010, 22 February 2011, 13 June 2011, 23 December 2011 and related aftershocks.

The scope of work included:

- Assessment of the nature and extent of the building damage.
- Visual assessment of the building strength particularly with respect to safety of occupants if the building is currently occupied.
- Assessment of requirements for detailed engineering evaluation including geotechnical investigation, level survey and any areas where linings and floor coverings need removal to expose structural damage.

This report outlines the results of our Qualitative Assessment of damage to the Fencing Centre and is based on the Detailed Engineering Evaluation Procedure document issued by the Structural Advisory Group on 19 July 2011, visual inspections, available structural documentation and summary calculations as appropriate.

2 Description of the Building

2.1 Building Age and Configuration

Built in 1970's the Fencing Centre is single storey warehouse style building. The building has a steel roof, reinforced concrete masonry perimeter walls, steel portal frames and a concrete slab on ground with strip foundations. The approximate floor area of the building is 664 square metres. It is an importance level 2 structure in accordance with NZS 1170 Part 0:2002.

2.2 Building Structural Systems Vertical and Horizontal

The Fencing Centre has well defined load paths. The south-west half of the steel roof is supported on steel portal frames that transfer loads to the foundation and the roof of the north-east half of the structure is supported on block work walls. Lateral loads are resisted by portal frames in the northwest - southeast direction. The southwest half of the building has concrete block work walls around the perimeter surrounding the steel frames and the northeast half mainly consists of concrete block work as the main structural system. Internally the ceiling is lined with plaster board. Externally above the concrete block work walls the area is clad with corrugated iron.

2.3 Reference Building Type

The Fencing Centre is a basic warehouse typical of its age and style. It is assumed that it was not subject to specific engineering design; rather it was constructed to a reliable formula known to achieve the performance and aesthetic objectives of the time it was built.

2.4 Building Foundation System and Soil Conditions

The Fencing Centre, as discussed above, has a concrete slab on ground. It is likely that there are strip footings under the concrete block work walls. The land and surrounds of Fencing Centre is zoned TC2 therefore minor to moderate land damage from liquefaction is possible in future large earthquakes. Additionally there are no signs in the vicinity of Fencing Centre of liquefaction bulges or boils and subsidence.

2.5 Available Structural Documentation and Inspection Priorities

No architectural or structural drawings were available for the Fencing Centre. Inspection priorities related to a review of potential damage to foundations and consideration of wall bracing adequacy. The generic building type for the Fencing Centre is a 1970s concrete block work and portal framed warehouse and this type of structure has performed fairly well during the Canterbury Earthquakes.

2.6 Available Survey Information

A floor levels survey was undertaken to establish the level of unevenness. The results of the survey are presented on the attached drawing in Appendix A. All of the levels were taken on top of the existing floor coverings which may have introduced some margin of error.

The Department of Building and Housing (DBH) published the “Revised Guidance on Repairing and Rebuilding Houses Affected by the Canterbury Earthquake Sequence” in November 2011, which recommends some form of re-levelling or rebuilding of the floor

1. If the slope is greater than 0.5% for any two points more than 2m apart, or
2. If the variation in level over the floor plan is greater than 50mm, or
3. If there is significant cracking of the floor.

It is important to note that these figures are recommendations and are only intended to be applied to residential buildings. However, they provide useful guidance in determining acceptable floor level variations.

The floor levels for the Fencing Centre were found to be within the recommended tolerances excluding the floor area between the hall and the toilets. The large slopes that are shown on the level survey are around cracks in the slab.

3 Structural Investigation

3.1 Summary of Building Damage

The Fencing Centre is currently in use and was occupied at the time the damage assessment was carried out.

The Fencing Centre has performed well and excluding the slab has only suffered minor cosmetic damage. There was some minor cracking in masonry walls and a small area of the slab had large variations in levels and significant cracking.

3.2 Record of Intrusive Investigation

The extent of damage was relatively minor except for a small section of the slab and therefore, an intrusive investigation is recommended for the Fencing Centre. The intrusive investigation should remove the carpet to observe the extent of the damage to the slab. The external walls were scanned for reinforcement and steel was detected at 800mm centres.

3.3 Damage Discussion

There was only minor observed damage to the Fencing Centre structure as a result of seismic actions. Damage to the wall is a common occurrence in this type of construction and occurs as concrete masonry is relatively brittle causing cracks to develop with a limited amount of movement. The slab damage is likely to be from ground settlement caused by the earthquakes. The slab damage is not likely to affect the lateral or gravity load resistance systems of the structure.

4 Building Review Summary

4.1 Building Review Statement

Because of the generic nature of the building a significant amount of information can be inferred from an external and internal inspection. Steel was detected by a scanner in the external walls at 800mm centres.

4.2 Critical Structural Weaknesses

No specific critical structural weaknesses were identified as part of the building qualitative assessment.

5 Building Strength (Refer to Appendix C for background information)

5.1 General

The Fencing Centre is, as discussed above, a typical example of its generic style, 1970's warehouses. It is of a type of building that, due to its high lateral load resistance of its reinforced masonry walls and ductility of its steel portal frames, has typically performed well. The Fencing Centre is not an exception to this. It has performed well and there is only minor damage to the building related to the recent earthquakes.

5.2 Initial %NBS Assessment

It is assumed that the Fencing Centre may likely have been subject to specific engineering design and the initial evaluation procedure or IEP may be an appropriate method of assessment for this building. Nevertheless an estimate of lateral load capacity can be made by adopting assumed values for strengths of existing materials and calculating the capacity of existing walls.

Selected assessment seismic parameters are tabulated in the Table 1 below.

Table 1: Parameters used in the Seismic Assessment

Seismic Parameter	Quantity	Comment/Reference
Site Soil Class	D	NZS 1170.5:2004, Clause 3.1.3, Deep or Soft Soil
Site Hazard Factor, Z	0.30	DBH Info Sheet on Seismicity Changes (Effective 19 May 2011)
Return period Factor, R_u	1.00	NZS 1170.5:2004, Table 3.5
Ductility Factor in Transverse Direction, μ	2.00	Steel moment frames and blockwork walls on perimeter and north-east half of the building
Ductility Factor in Longitudinal Direction, μ	1.25	Blockwork walls on perimeter

The seismic demand for the Fencing Centre has been calculated based on the current code requirements of NZS 1170.5:2004. The capacity of the existing walls in the building was calculated from assumed strengths of existing materials and the number and length of walls present for both the northwest – southeast and northeast – southwest directions. The seismic demand was then compared with the building capacity in these directions. The building was found to have a sufficient number and length of walls in both the northwest – southeast and northeast – southwest directions to achieve a capacity greater than 100% NBS. It has been assumed that bracing has been provided in the ceiling in such a manner that the portals only take loads in the transverse direction and that the roof loads are transferred to the walls evenly throughout the structure. Another assumption is that the portals will take the roof load above their span without interaction with the blockwork walls.

5.3 Results Discussion

Basic analysis shows that the Fencing Centre is capable of achieving seismic performance in line with the current code requirements. This is not surprising as there are a large number of well distributed walls providing seismic resistance. The buildings torsional performance is likely to be worse than other buildings of more uniform materials as the blockwork walls are very stiff but the portals are quite flexible.

6 Conclusions and Recommendations

There is no clear evidence of any liquefaction or ground movement in the vicinity of the Fencing Centre although there is damage to one portion of the floor slab therefore **a geotechnical investigation is currently considered necessary**. It is also recommended to perform an intrusive investigation on that section of the floor slab to determine the extent of the damage.

The building is currently occupied and in use and in our opinion the Fencing Centre **is considered suitable for continued occupation**.

7 Explanatory Statement

The inspections of the building discussed in this report have been undertaken to assess structural earthquake damage. No analysis has been undertaken to assess the strength of the building or to determine whether or not it complies with the relevant building codes, except to the extent that Aurecon expressly indicates otherwise in the report. Aurecon has not made any assessment of structural stability or building safety in connection with future aftershocks or earthquakes – which have the potential to damage the building and to jeopardise the safety of those either inside or adjacent to the building, except to the extent that Aurecon expressly indicates otherwise in the report.

This report is necessarily limited by the restricted ability to carry out inspections due to potential structural instabilities/safety considerations, and the time available to carry out such inspections. The report does not address defects that are not reasonably discoverable on visual inspection, including defects in inaccessible places and latent defects. Where site inspections were made, they were restricted to external inspections and, where practicable, limited internal visual inspections.

No drawings were obtained from the Christchurch City Council records.

While this report may assist the client in assessing whether the building should be strengthened, that decision is the sole responsibility of the client.

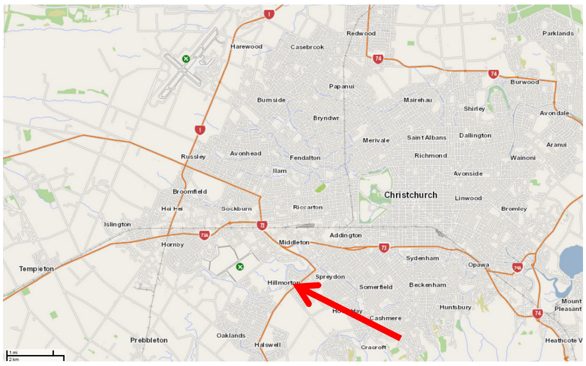
This review has been prepared by Aurecon at the request of its client and is exclusively for the client's use. It is not possible to make a proper assessment of this review without a clear understanding of the terms of engagement under which it has been prepared, including the scope of the instructions and directions given to and the assumptions made by Aurecon. The report will not address issues which would need to be considered for another party if that party's particular circumstances, requirements and experience were known and, further, may make assumptions about matters of which a third party is not aware. No responsibility or liability to any third party is accepted for any loss or damage whatsoever arising out of the use of or reliance on this report by any third party.

Without limiting any of the above, Aurecon's liability, whether under the law of contract, tort, statute, equity or otherwise, is limited as set out in the terms of the engagement with the client.

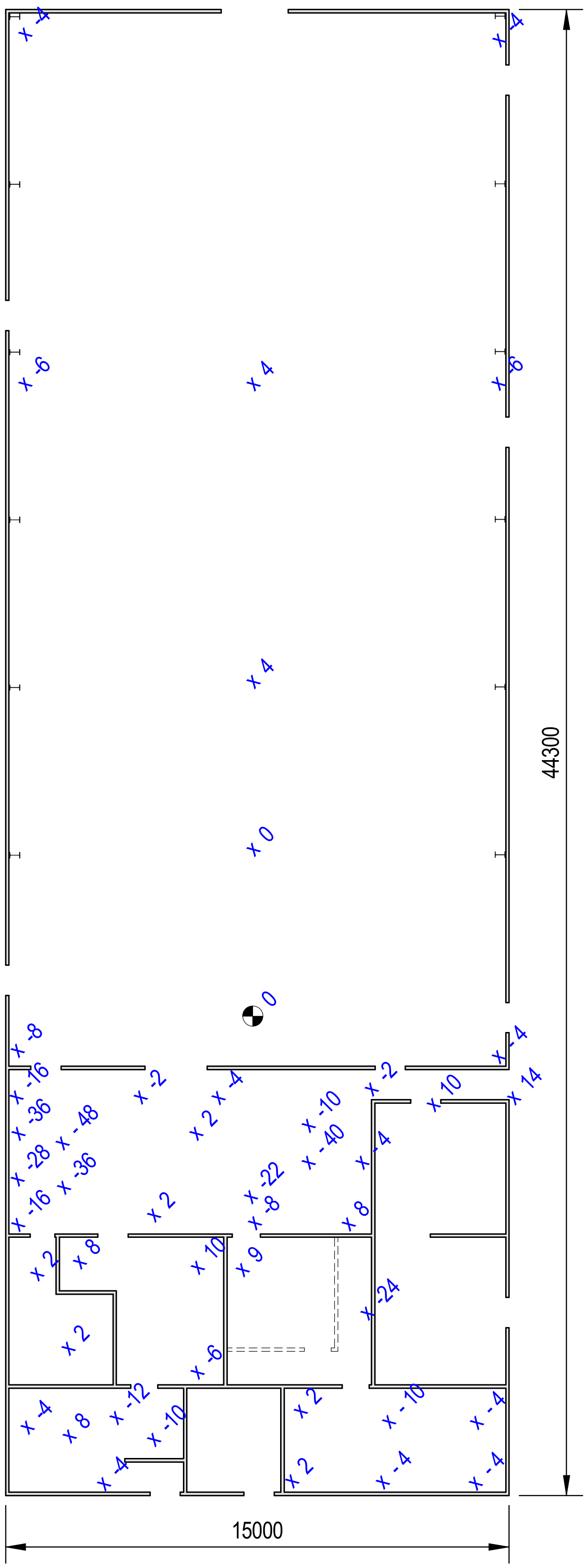
Appendices

Appendix A
Site Map, Photos and Levels Survey Results

2 March 2012 – Fencing Centre Site Photographs

Location of the Fencing Centre	
Aerial photograph of Little River Service Centre	
Building northern elevation	

Building western elevation	
Building eastern elevation	
Internal view	



PLAN - LEVELS

1 : 200

3/08/2012 3:37:25 p.m.

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CLIENT
Christchurch City Council

REV	DATE	REVISION DETAILS	APPROVAL
A	03.08.12	A	APP

DRAWN	DESIGNED
N.Stanojevic	S.Edwards

CHECKED: Checker

APPROVED: Approver — DATE

PROJECT: FENCING CENTRE
95 JACK HINTO DRIVE

TITLE: PLAN - LEVELS

PRELIMINARY
NOT FOR CONSTRUCTION

PROJECT No.		
228362		
SCALE	SIZE	
1 : 200	A4	
DRAWING No.	REV	
S-01-02	A	

Appendix B
References

1. Department of Building and Housing (DBH), “Revised Guidance on Repairing and Rebuilding Houses Affected by the Canterbury Earthquake Sequence”, November 2011

2. New Zealand Society for Earthquake Engineering (NZSEE), “Assessment and Improvement of the Structural Performance of Buildings in Earthquakes”, April 2012

3. Standards New Zealand, “AS/NZS 1170 Part 0, Structural Design Actions: General Principles”, 2002

4. Standards New Zealand, “AS/NZS 1170 Part 1, Structural Design Actions: Permanent, imposed and other actions”, 2002

5. Standards New Zealand, “NZS 1170 Part 5, Structural Design Actions: Earthquake Actions – New Zealand”, 2004

6. Standards New Zealand, “NZS 3101 Part 1, The Design of Concrete Structures”, 2006

7. Standards New Zealand, “NZS 3404 Part 1, Steel Structures Standard”, 1997

8. Standards New Zealand, “NZS 3606, Timber Structures Standard”, 1993

9. Standards New Zealand, “NZS 3604, Timber Framed Structures”, 2011

10. Standards New Zealand, “NZS 4229, Concrete Masonry Buildings Not Requiring Specific Engineering Design”, 1999

11. Standards New Zealand, “NZS 4230, Design of Reinforced Concrete Masonry Structures”, 2004

Appendix C
Strength Assessment Explanation

New building standard (NBS)

New building standard (NBS) is the term used with reference to the earthquake standard that would apply to a new building of similar type and use if the building was designed to meet the latest design Codes of Practice. If the strength of a building is less than this level, then its strength is expressed as a percentage of NBS.

Earthquake Prone Buildings

A building can be considered to be earthquake prone if its strength is less than one third of the strength to which an equivalent new building would be designed, that is, less than 33%NBS (as defined by the New Zealand Building Act). If the building strength exceeds 33%NBS but is less than 67%NBS the building is considered at risk.

Christchurch City Council Earthquake Prone Building Policy 2010

The Christchurch City Council (CCC) already had in place an Earthquake Prone Building Policy (EPB Policy) requiring all earthquake-prone buildings to be strengthened within a timeframe varying from 15 to 30 years. The level to which the buildings were required to be strengthened was 33%NBS.

As a result of the 4 September 2010 Canterbury earthquake the CCC raised the level that a building was required to be strengthened to from 33% to 67% NBS but qualified this as a target level and noted that the actual strengthening level for each building will be determined in conjunction with the owners on a building-by-building basis. Factors that will be taken into account by the Council in determining the strengthening level include the cost of strengthening, the use to which the building is put, the level of danger posed by the building, and the extent of damage and repair involved.

Irrespective of strengthening level, the threshold level that triggers a requirement to strengthen is 33%NBS.

As part of any building consent application fire and disabled access provisions will need to be assessed.

Christchurch Seismicity

The level of seismicity within the current New Zealand loading code (AS/NZS 1170) is related to the seismic zone factor. The zone factor varies depending on the location of the building within NZ. Prior to the 22nd February 2011 earthquake the zone factor for Christchurch was 0.22. Following the earthquake the seismic zone factor (level of seismicity) in the Christchurch and surrounding areas has been increased to 0.3. This is a 36% increase.

For this assessment, the building's earthquake resistance is compared with the current New Zealand Building Code requirements for a new building constructed on the site. This is expressed as a percentage of new building standard (%NBS). The new building standard load requirements have been determined in accordance with the current earthquake loading standard (NZS 1170.5:2004 Structural design actions - Earthquake actions - New Zealand).

The likely capacity of this building has been derived in accordance with the New Zealand Society for Earthquake Engineering (NZSEE) guidelines 'Assessment and Improvement of the Structural Performance of Buildings in Earthquakes' (AISPBE), 2006. These guidelines provide an Initial Evaluation Procedure that assesses a buildings capacity based on a comparison of loading codes from when the building was designed and currently. It is a quick high-level procedure that can be used when undertaking a Qualitative analysis of a building. The guidelines also provide guidance on calculating a modified Ultimate Limit State capacity of the building which is much more accurate and can be used when undertaking a Quantitative analysis.

The New Zealand Society for Earthquake Engineering has proposed a way for classifying earthquake risk for existing buildings in terms of %NBS and this is shown in Figure C1 below.

Description	Grade	Risk	%NBS	Existing Building Structural Performance	Improvement of Structural Performance	
					Legal Requirement	NZSEE Recommendation
Low Risk Building	A or B	Low	Above 67	Acceptable (improvement may be desirable)	The Building Act sets no required level of structural improvement (unless change in use) This is for each TA to decide. Improvement is not limited to 34%NBS.	100%NBS desirable. Improvement should achieve at least 67%NBS
Moderate Risk Building	B or C	Moderate	34 to 66	Acceptable legally. Improvement recommended		Not recommended. Acceptable only in exceptional circumstances
High Risk Building	D or E	High	33 or lower	Unacceptable (Improvement	Unacceptable	Unacceptable

Figure C1: NZSEE Risk Classifications Extracted from table 2.2 of the NZSEE 2006 AISPBE Guidelines

Table C1 below compares the percentage NBS to the relative risk of the building failing in a seismic event with a 10% probability of exceedance in 50 years (i.e. 0.2% in the next year). It is noted that the current seismic risk in Christchurch results in a 6% probability of exceedance in the next year.

Table C1: Relative Risk of Building Failure In A

Percentage of New Building Standard (%NBS)	Relative Risk (Approximate)
>100	<1 time
80-100	1-2 times
67-80	2-5 times
33-67	5-10 times
20-33	10-25 times
<20	>25 times

Appendix D
Background and Legal Framework

Background

Aurecon has been engaged by the Christchurch City Council (CCC) to undertake a detailed engineering evaluation of the building

This report is a Qualitative Assessment of the building structure, and is based on the Detailed Engineering Evaluation Procedure document (draft) issued by the Structural Advisory Group on 19 July 2011.

A qualitative assessment involves inspections of the building and a desktop review of existing structural and geotechnical information, including existing drawings and calculations, if available.

The purpose of the assessment is to determine the likely building performance and damage patterns, to identify any potential critical structural weaknesses or collapse hazards, and to make an initial assessment of the likely building strength in terms of percentage of new building standard (%NBS).

At the time of this report, no intrusive site investigation, detailed analysis, or modelling of the building structure had been carried out. Construction drawings were made available, and these have been considered in our evaluation of the building. The building description below is based on a review of the drawings and our visual inspections.

Compliance

This section contains a brief summary of the requirements of the various statutes and authorities that control activities in relation to buildings in Christchurch at present.

Canterbury Earthquake Recovery Authority (CERA)

CERA was established on 28 March 2011 to take control of the recovery of Christchurch using powers established by the Canterbury Earthquake Recovery Act enacted on 18 April 2011. This act gives the Chief Executive Officer of CERA wide powers in relation to building safety, demolition and repair. Two relevant sections are:

Section 38 – Works

This section outlines a process in which the chief executive can give notice that a building is to be demolished and if the owner does not carry out the demolition, the chief executive can commission the demolition and recover the costs from the owner or by placing a charge on the owners' land.

Section 51 – Requiring Structural Survey

This section enables the chief executive to require a building owner, insurer or mortgagee carry out a full structural survey before the building is re-occupied.

We understand that CERA will require a detailed engineering evaluation to be carried out for all buildings (other than those exempt from the Earthquake Prone Building definition in the Building Act). It is anticipated that CERA will adopt the Detailed Engineering Evaluation Procedure document (draft) issued by the Structural Advisory Group on 19 July 2011. This document sets out a methodology for both qualitative and quantitative assessments.

The qualitative assessment is a desk-top and site inspection assessment. It is based on a thorough visual inspection of the building coupled with a review of available documentation such as drawings and specifications. The quantitative assessment involves analytical calculation of the buildings strength and may require non-destructive or destructive material testing, geotechnical testing and intrusive investigation.

It is anticipated that factors determining the extent of evaluation and strengthening level required will include:

- The importance level and occupancy of the building
- The placard status and amount of damage
- The age and structural type of the building
- Consideration of any critical structural weaknesses
- The extent of any earthquake damage

Building Act

Several sections of the Building Act are relevant when considering structural requirements:

Section 112 – Alterations

This section requires that an existing building complies with the relevant sections of the Building Code to at least the extent that it did prior to any alteration. This effectively means that a building cannot be weakened as a result of an alteration (including partial demolition).

Section 115 – Change of Use

This section requires that the territorial authority (in this case Christchurch City Council (CCC)) be satisfied that the building with a new use complies with the relevant sections of the Building Code 'as near as is reasonably practicable'. Regarding seismic capacity 'as near as reasonably practicable' has previously been interpreted by CCC as achieving a minimum of 67%NBS however where practical achieving 100%NBS is desirable. The New Zealand Society for Earthquake Engineering (NZSEE) recommend a minimum of 67%NBS.

Section 121 – Dangerous Buildings

The definition of dangerous building in the Act was extended by the Canterbury Earthquake (Building Act) Order 2010, and it now defines a building as dangerous if:

- in the ordinary course of events (excluding the occurrence of an earthquake), the building is likely to cause injury or death or damage to other property; or
- in the event of fire, injury or death to any persons in the building or on other property is likely because of fire hazard or the occupancy of the building; or
- there is a risk that the building could collapse or otherwise cause injury or death as a result of earthquake shaking that is less than a 'moderate earthquake' (refer to Section 122 below); or
- there is a risk that that other property could collapse or otherwise cause injury or death; or
- a territorial authority has not been able to undertake an inspection to determine whether the building is dangerous.

Section 122 – Earthquake Prone Buildings

This section defines a building as earthquake prone if its ultimate capacity would be exceeded in a 'moderate earthquake' and it would be likely to collapse causing injury or death, or damage to other property. A moderate earthquake is defined by the building regulations as one that would generate ground shaking 33% of the shaking used to design an equivalent new building.

Section 124 – Powers of Territorial Authorities

This section gives the territorial authority the power to require strengthening work within specified timeframes or to close and prevent occupancy to any building defined as dangerous or earthquake prone.

Section 131 – Earthquake Prone Building Policy

This section requires the territorial authority to adopt a specific policy for earthquake prone, dangerous and insanitary buildings.

Christchurch City Council Policy

Christchurch City Council adopted their Earthquake Prone, Dangerous and Insanitary Building Policy in 2006. This policy was amended immediately following the Darfield Earthquake of the 4th September 2010.

The 2010 amendment includes the following:

- A process for identifying, categorising and prioritising Earthquake Prone Buildings, commencing on 1 July 2012;
- A strengthening target level of 67% of a new building for buildings that are Earthquake Prone;
- A timeframe of 15-30 years for Earthquake Prone Buildings to be strengthened; and,
- Repair works for buildings damaged by earthquakes will be required to comply with the above.

The council has stated their willingness to consider retrofit proposals on a case by case basis, considering the economic impact of such a retrofit.

We anticipate that any building with a capacity of less than 33%NBS (including consideration of critical structural weaknesses) will need to be strengthened to a target of 67%NBS of new building standard as recommended by the Policy.

If strengthening works are undertaken, a building consent will be required. A requirement of the consent will require upgrade of the building to comply ‘as near as is reasonably practicable’ with:

- The accessibility requirements of the Building Code.
- The fire requirements of the Building Code. This is likely to require a fire report to be submitted with the building consent application.

Building Code

The building code outlines performance standards for buildings and the Building Act requires that all new buildings comply with this code. Compliance Documents published by The Department of Building and Housing can be used to demonstrate compliance with the Building Code.

After the February Earthquake, on 19 May 2011, Compliance Document B1: Structure was amended to include increased seismic design requirements for Canterbury as follows:

- Hazard Factor increased from 0.22 to 0.3 (36% increase in the basic seismic design load)
- Serviceability Return Period Factor increased from 0.25 to 0.33 (80% increase in the serviceability design loads when combined with the Hazard Factor increase)

The increase in the above factors has resulted in a reduction in the level of compliance of an existing building relative to a new building despite the capacity of the existing building not changing.

Appendix E
Standard Reporting Spread Sheet

Detailed Engineering Evaluation Summary Data V1.11

Location

Building Name: Fencing centre
Building Address: Unit: No: 95 Street: Jack Hinton Drive
Legal Description:
GPS south: Degrees 43, Min 32, Sec 40.60
GPS east: Degrees 172, Min 36, Sec 19.48
Building Unique Identifier (CCC): PRO 2394 B003

Reviewer: Lee Howard
CPEng No: 1000889
Company: Aurecon
Company project number: 228602
Company phone number: 03 375 0761
Date of submission: 1/07/2013
Inspection Date: 2/03/2012
Revision: 2
Is there a full report with this summary? yes

Site

Site slope: flat
Soil type: mixed
Site Class (to NZS1170.5): D
Proximity to waterway (m, if <100m):
Proximity to clifftop (m, if < 100m):
Proximity to cliff base (m,if <100m):

Max retaining height (m):
Soil Profile (if available):
If Ground improvement on site, describe:
Approx site elevation (m): 10.00

Building

No. of storeys above ground: 1 (single storey = 1)
Ground floor split? no
Storeys below ground: 0
Foundation type: strip footings
Building height (m): 3.00
Floor footprint area (approx): 664
Age of Building (years): 40

Ground floor elevation (Absolute) (m): 10.00
Ground floor elevation above ground (m): 10.10
if Foundation type is other, describe:
height from ground to level of uppermost seismic mass (for IEP only) (m):
Date of design: 1965-1976

Strengthening present? no
Use (ground floor): public
Use (upper floors):
Use notes (if required):
Importance level (to NZS1170.5): IL2

If so, when (year)?
And what load level (%g)?
Brief strengthening description:

Gravity Structure

Gravity System: frame system
Roof: steel framed — rafter type, purlin type and cladding: Steel Radters,Steel Purlins, Metal Roofing
Floors: concrete flat slab — slab thickness (mm):
Beams: steel non-composite — beam and connector type: 250UB31.4
Columns: structural steel — typical dimensions (mm x mm): 250UB31.4
Walls: partially filled concrete masonry — thickness (mm): 190

Lateral load resisting structure

Lateral system along: other (note) — describe system: reinforced blockwork walls
Ductility assumed, μ: 1.25
Period along: 0.40 — 0.00 — estimate or calculation? estimated
Total deflection (ULS) (mm): 35 — estimate or calculation? estimated
maximum interstorey deflection (ULS) (mm): 35 — estimate or calculation? estimated

Note: Define along and across in detailed report!

Lateral system across: other (note) — describe system: Mixed System, reinforced blockwork Walls and steel portal Framed Walls
Ductility assumed, μ: 2.00
Period across: 0.40 — 0.00 — estimate or calculation? estimated
Total deflection (ULS) (mm): 35 — estimate or calculation? estimated
maximum interstorey deflection (ULS) (mm): 35 — estimate or calculation? estimated

Separations:

north (mm):
east (mm):
south (mm):
west (mm):

leave blank if not relevant

Non-structural elements

Stairs:
Wall cladding:
Roof Cladding: Metal
Glazing: aluminium frames
Ceilings: plaster, fixed
Services(list):

describe

Available documentation

Architectural — original designer name/date:
Structural — original designer name/date:
Mechanical — original designer name/date:
Electrical — original designer name/date:
Geotech report — original designer name/date:

Damage

Site:
(refer DEE Table 4-2)

Site performance: — Describe damage: minor - none
Settlement: none observed — notes (if applicable):
Differential settlement: none observed — notes (if applicable):
Liquefaction: none apparent — notes (if applicable):
Lateral Spread: none apparent — notes (if applicable):
Differential lateral spread: none apparent — notes (if applicable):
Ground cracks: none apparent — notes (if applicable):
Damage to area: none apparent — notes (if applicable):

Building:

Current Placard Status: green

Along — Damage ratio: 0% — Describe how damage ratio arrived at: Qualitataive judgement
Describe (summary):

Across — Damage ratio: 0%
Describe (summary):

$$Damage\_Ratio = \frac{(\%NBS(before) - \%NBS(after))}{\%NBS(before)}$$

Diaphragms — Damage?: no — Describe:
CSWs: — Damage?: no — Describe:
Pounding: — Damage?: no — Describe:
Non-structural: — Damage?: no — Describe:

Recommendations

Level of repair/strengthening required: none — Describe:
Building Consent required: — Describe:
Interim occupancy recommendations: full occupancy — Describe:

Along — Assessed %NBS before: 100% ##### %NBS from IEP below
Assessed %NBS after: 100%

If IEP not used, please detail assessment methodology: Analysis of Capacity and Demand

Across — Assessed %NBS before: 100% ##### %NBS from IEP below
Assessed %NBS after: 100%

IEP **Use of this method is not mandatory - more detailed analysis may give a different answer, which would take precedence. Do not fill in fields if not using IEP.**

Period of design of building (from above): 1965-1976
h_n from above: m

Seismic Zone, if designed between 1965 and 1992:

not required for this age of building
not required for this age of building

	along	across
Period (from above):	0.4	0.4
(%NBS)nom from Fig 3.3:		

Note:1 for specifically design public buildings, to the code of the day: pre-1965 = 1.25; 1965-1976, Zone A =1.33; 1965-1976, Zone B = 1.2; all else 1.0 — 1.00
Note 2: for RC buildings designed between 1976-1984, use 1.2 — 1.0
Note 3: for buildngs designed prior to 1935 use 0.8, except in Wellington (1.0) — 1.0

	along	across
Final (%NBS)nom:	**0%**	**0%**

2.2 Near Fault Scaling Factor

	along	across
Near Fault scaling factor, from NZS1170.5, cl 3.1.6:		1.00
Near Fault scaling factor (1/N(T,D), **Factor A**:	1	1

2.3 Hazard Scaling Factor

Hazard factor Z for site from AS1170.5, Table 3.3:	
Z_{1992}, from NZS4203:1992	
Hazard scaling factor, **Factor B:**	#DIV/0!

2.4 Return Period Scaling Factor

Building Importance level (from above):	2
Return Period Scaling factor from Table 3.1, **Factor C**:	

2.5 Ductility Scaling Factor

	along	across
Assessed ductility (less than max in Table 3.2)	1.00	1.00
Ductility scaling factor: =1 from 1976 onwards; or =kμ, if pre-1976, fromTable 3.3:		
Ductiity Scaling Factor, **Factor D**:	0.00	0.00

2.6 Structural Performance Scaling Factor:

	along	across
Sp:	1.000	1.000
Structural Performance Scaling Factor **Factor E**:	1	1

2.7 Baseline %NBS, (NBS%)b = (%NBS)nom x A x B x C x D x E

	along	across
%NBSb:	**#DIV/0!**	**#DIV/0!**

Global Critical Structural Weaknesses: (refer to NZSEE IEP Table 3.4)

3.1. Plan Irregularity, factor A: 1

3.2. Vertical irregularity, Factor B: 1

3.3. Short columns, Factor C: 1

3.4. Pounding potential

Pounding effect D1, from Table to right 1.0

Height Difference effect D2, from Table to right 1.0

Therefore, Factor D: 1

3.5. Site Characteristics 1

Table for selection of D1	Severe	Significant	Insignificant/none
Separation	0<sep<.005H	.005<sep<.01H	Sep>.01H
Alignment of floors within 20% of H	**0.7**	**0.8**	**1**
Alignment of floors not within 20% of H	**0.4**	**0.7**	**0.8**

Table for Selection of D2	Severe	Significant	Insignificant/none
Separation	0<sep<.005H	.005<sep<.01H	Sep>.01H
Height difference > 4 storeys	0.4	0.7	1
Height difference 2 to 4 storeys	0.7	0.9	1
Height difference < 2 storeys	1	1	1

3.6. Other factors, Factor F

	Along	Across
For ≤ 3 storeys, max value =2.5, otherwise max valule =1.5, no minimum		
Rationale for choice of F factor, if not 1		

Detail Critical Structural Weaknesses: (refer to DEE Procedure section 6)

List any: Refer also section 6.3.1 of DEE for discussion of F factor modification for other critical structural weaknesses

3.7. Overall Performance Achievement ratio (PAR)

	along	across
	0.00	**0.00**

4.3 PAR x (%NBS)b:

	along	across
PAR x Baselline %NBS:	**#DIV/0!**	**#DIV/0!**

4.4 Percentage New Building Standard (%NBS), (before) **#DIV/0!**

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