



Canterbury Agricultural Park  
Toilets  
Qualitative Engineering Evaluation

**Reference:** 231550  
**Prepared for:**  
Christchurch City Council

Functional Location ID: PRK 2412 BLDG 002

**Revision:** 2

Address: 61 Wigram Road

**Date:** 5 July 2013

# Document Control Record

Document prepared by:

Aurecon New Zealand Limited  
 Level 2, 518 Colombo Street  
 Christchurch 8011  
 PO Box 1061  
 Christchurch 8140  
 New Zealand

**T** +64 3 366 0821  
**F** +64 3 379 6955  
**E** christchurch@aurecongroup.com  
**W** aurecongroup.com

A person using Aurecon documents or data accepts the risk of:

- Using the documents or data in electronic form without requesting and checking them for accuracy against the original hard copy version.
- Using the documents or data for any purpose not agreed to in writing by Aurecon.

Document control				aurecon		
<b>Report Title</b>		Qualitative Engineering Evaluation				
<b>Functional Location ID</b>		<b>PRK 2412 BLDG 002</b>	<b>Project Number</b>		231550	
<b>File Path</b>		P:\ 231550 - Canterbury Agricultural Park Toilets.docx				
<b>Client</b>		Christchurch City Council	<b>Client Contact</b>		Michael Sheffield	
<b>Rev</b>	<b>Date</b>	<b>Revision Details/Status</b>	<b>Prepared</b>	<b>Author</b>	<b>Verifier</b>	<b>Approver</b>
1	15 January 2013	Draft	C. Bong	C. Bong	L. Castillo	L. Castillo
2	5 July 2013	Final	C. Bong	C. Bong	L. Castillo	L. Castillo
<b>Current Revision</b>		2				

Approval			
Author Signature		Approver Signature	
Name	Christopher Bong	Name	Luis Castillo
Title	Structural Engineer	Title	Senior Structural Engineer



# Contents

<b>Executive Summary</b>	<b>1</b>
<b>1 Introduction</b>	<b>2</b>
1.1 General	2
<b>2 Description of the Building</b>	<b>2</b>
2.1 Building Age and Configuration	2
2.2 Building Structural Systems Vertical and Horizontal	2
2.3 Reference Building Type	3
2.4 Building Foundation System and Soil Conditions	3
2.5 Available Structural Documentation and Inspection Priorities	3
2.6 Available Survey Information	3
<b>3 Structural Investigation</b>	<b>4</b>
3.1 Summary of Building Damage	4
3.2 Record of Intrusive Investigation	4
3.3 Damage Discussion	4
<b>4 Building Review Summary</b>	<b>4</b>
4.1 Building Review Statement	4
4.2 Critical Structural Weaknesses	4
<b>5 Building Strength (Refer to Appendix C for background information)</b>	<b>5</b>
5.1 General	5
5.2 Initial %NBS Assessment	5
5.3 Results Discussion	6
<b>6 Conclusions and Recommendations</b>	<b>6</b>
<b>7 Explanatory Statement</b>	<b>6</b>

## Appendices

**Appendix A Site Location, Photos and Levels Survey**

**Appendix B References**


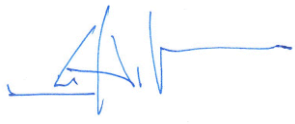
**Appendix C Strength Assessment Explanation**

**Appendix D Background and Legal Framework**

**Appendix E Standard Reporting Spread Sheet**

# Executive Summary

This is a summary of the Qualitative Engineering Evaluation for the Canterbury Agricultural Park Toilets building and is based on the Detailed Engineering Evaluation Procedure document issued by the Engineering Advisory Group on 19 July 2011, visual inspections, available structural documentation and summary calculations as appropriate.

<b>Building Details</b>	<b>Name</b>	Canterbury Agricultural Park Toilets			
<b>Building Location ID</b>	PRK 2412 BLDG 002	<b>Multiple Building Site</b>	Y		
<b>Building Address</b>	61 Wigram Road (access off Curletts Road)	<b>No. of residential units</b>	0		
<b>Soil Technical Category</b>	NA	<b>Importance Level</b>	1	<b>Approximate Year Built</b>	1990s
<b>Foot Print (m<sup>2</sup>)</b>	13	<b>Storeys above ground</b>	1	<b>Storeys below ground</b>	0
<b>Type of Construction</b>	Lightweight profiled sheet metal roof, timber purlins and rafters, vertical board and batten walls and concrete slab on grade foundations.				
<b>Qualitative L4 Report Results Summary</b>					
<b>Building Occupied</b>	Y	The Canterbury Agricultural Park Toilets is currently in service.			
<b>Suitable for Continued Occupancy</b>	Y	The Canterbury Agricultural Park Toilets is suitable for continued use.			
<b>Key Damage Summary</b>	Y	Refer to summary of building damage Section 3.1 report body.			
<b>Critical Structural Weaknesses (CSW)</b>	N	No critical structural weaknesses were identified.			
<b>Levels Survey Results</b>	Y	The maximum slope was found to be 2.3% or 1 in 43 which is acceptable despite being outside the Department of Building and Housing Guidelines.			
<b>Building %NBS From Analysis</b>	>100%	Based on an analysis of bracing capacity and demand.			
<b>Qualitative L4 Report Recommendations</b>					
<b>Geotechnical Survey Required</b>	N	Geotechnical survey not required due to lack of observed ground damage on site.			
<b>Proceed to L5 Quantitative DEE</b>	N	A quantitative DEE is not required for this structure.			
<b>Approval</b>					
<b>Author Signature</b>		<b>Approver Signature</b>			
<b>Name</b>	Christopher Bong	<b>Name</b>	Luis Castillo		
<b>Title</b>	Structural Engineer	<b>Title</b>	Senior Structural Engineer		



# 1 Introduction

## 1.1 General

On 21 August 2012 Aurecon engineers visited the Canterbury Agricultural Park Toilets to undertake a qualitative building damage assessment on behalf of Christchurch City Council. Detailed visual inspections were carried out to assess the damage caused by the Canterbury earthquake sequence

The scope of work included:

- Assessment of the nature and extent of the building damage.
- Visual assessment of the building strength particularly with respect to safety of occupants if the building is currently occupied.
- Assessment of requirements for detailed engineering evaluation including geotechnical investigation, level survey and any areas where linings and floor coverings need removal to expose structural damage.

This report outlines the results of our Qualitative Assessment of damage to the Canterbury Agricultural Park Toilets and is based on the Detailed Engineering Evaluation Procedure document issued by the Engineering Advisory Group on 19 July 2011, visual inspections, available structural documentation and summary calculations as appropriate.

## 2 Description of the Building

### 2.1 Building Age and Configuration

The Canterbury Agricultural Park Toilets is single storey toilet block of recent construction, most likely in the mid 2010 based on the time lapse of aerial photographs made available through the Canterbury Geotechnical Database. The roof and wall structure of the building consists of lightweight timber frames. The exterior of the building is clad in a profiled sheet metal roof and vertical board and batten walls, while the interior has a plasterboard finish.

The building has an approximate floor area of 13 square metres. It is considered as an importance level 1 structure in accordance with AS/NZS 1170 Part 0:2002.

### 2.2 Building Structural Systems Vertical and Horizontal

The Canterbury Agricultural Park Toilets are of lightweight timber frame construction. The gravity loads from the timber framed roof are transferred into the ground via the timber framed walls and the concrete slab on grade foundations.

The lateral load resisting structure is identical to the gravity system in which the lateral loads in both principal directions are resisted by the lightweight timber framed walls.



## 2.3 Reference Building Type

The Canterbury Agricultural Park Toilets are of lightweight timber frame construction with lightweight wall and roof claddings. Buildings of this nature typically have a low seismic demand and a high level of ductility. These are two intrinsic qualities that have generally resulted in good seismic performance in the Canterbury earthquake sequence.

## 2.4 Building Foundation System and Soil Conditions

The Canterbury Agricultural Park Toilets are founded on a concrete slab on grade foundation.

The land of the Canterbury Agricultural Park has been zoned green by the Canterbury Earthquake Recovery Authority (CERA). As the land is currently non-residential, it has not been included in the Technical Category classification by the Building and Housing Group (formerly known as the Department of Building and Housing (DBH)) and has been classed as “N/A – Urban Non-residential”.

It is of note however, that the adjacent suburbs of Sockburn and Hornby to the north and east respectively consist of typically Technical Category 1 (TC1) land whilst Hillmorton and Halswell to the East and South consists of mainly of TC2 and traces of TC3 land.

## 2.5 Available Structural Documentation and Inspection Priorities

There were no drawings available for the Canterbury Agricultural Park Toilets.

The inspection priorities for the building are the review of damage to linings of the timber framed walls which are inherently weaker and more brittle than the timber framed walls. Additionally, the damage assessment focused on the building geometry and other forms of damage such as cracking in the concrete floor and jammed doors which may indicate seismic related subsidence.

## 2.6 Available Survey Information

A floor level survey was undertaken to establish the level of unevenness across the floors. The results of the survey are presented on the attached sketch in Appendix A.

The Building and Housing Group (formerly known as the DBH) published the “Revised Guidance on Repairing and Rebuilding Houses Affected by the Canterbury Earthquake Sequence” in November 2011, which recommends some form of re-levelling or rebuilding of the floor

1. If the slope is greater than 0.5% for any two points more than 2m apart, or
2. If the variation in level over the floor plan is greater than 50mm, or
3. If there is significant cracking of the floor.

It is important to note that these figures are recommendations and are only intended to be applied to residential buildings. However, they provide useful guidance in determining acceptable floor level variations.

The floor levels for the Canterbury Agricultural Park Toilets are considered to be acceptable despite not being within the recommended tolerances. This is because the toilet block floors have an in-built fall for drainage, which are often outside the DBH recommendations.



## 3 Structural Investigation

### 3.1 Summary of Building Damage

There was no damage noted in the damage assessment.

### 3.2 Record of Intrusive Investigation

There was no noted damage to the building and therefore, an intrusive investigation was neither warranted nor undertaken for Canterbury Agricultural Park Toilets.

### 3.3 Damage Discussion

No seismic related damage was noted in the damage assessment. This is not surprising given that the building is of recent, lightweight and ductile nature of construction

## 4 Building Review Summary

### 4.1 Building Review Statement

As noted above no intrusive investigations were carried out for the Canterbury Agricultural Park Toilets. Because of the generic nature of the building a significant amount of information can be inferred from an external and internal inspection.

### 4.2 Critical Structural Weaknesses

No specific critical structural weaknesses were identified as part of the building qualitative assessment.

## 5 Building Strength (Refer to Appendix C for background information)

### 5.1 General

The Canterbury Agricultural Park Toilets is of lightweight timber frame construction. The building has performed well in the Canterbury earthquake sequence due to its recent, lightweight and ductile construction as evidenced by the lack of damage noted in Section 3.

### 5.2 Initial %NBS Assessment

The Canterbury Agricultural Park Toilets has not been subjected to specific engineering design and the initial evaluation procedure or IEP is not an appropriate method of assessment for this building. Nevertheless an estimate of lateral load capacity can be made by adopting assumed values for strengths of existing materials and calculating the capacity of existing walls.

The selected assessment seismic parameters are tabulated in the table1 below.

Table 1: Parameters used in the Seismic Assessment

Seismic Parameter	Quantity	Comment/Reference
Site Soil Class	D	NZS 1170.5:2004, Clause 3.1.3, Deep or Soft Soil
Site Hazard Factor, Z	0.30	DBH Info Sheet on Seismicity Changes (Effective 19 May 2011)
Return period Factor, $R_u$	0.50	NZS 1170.5:2004, Table 3.5, Importance Level 1 Structure with a Design Life of 50 years
Ductility Factor in the Longitudinal Direction, $\mu$	3.00	Lightweight timber framed walls (AS1170.4 – 2007 Table 6.5A).
Ductility Factor in the Transverse Direction, $\mu$	3.00	Lightweight timber framed walls (AS1170.4 – 2007 Table 6.5A).

The seismic demand for the Canterbury Agricultural Park Toilets has been calculated based on the current code requirements of NZS 1170 Part 5:2004 (Structural Design Actions: Earthquake Actions – New Zealand). The capacity of the existing walls in the building was calculated from assumed strengths of existing materials and the number and length of walls present in each bracing line for both the longitudinal and transverse directions. The seismic demand was then compared with the building capacity in these directions. The building was found to have a sufficient number and length of walls in both the longitudinal and transverse directions to achieve a capacity in excess of 100% NBS.





### 5.3 Results Discussion

The bracing check is in agreement with the observations of the damage assessment. This is not surprising given that the building has an even distribution of walls that gives a torsional stable structure. Furthermore, the building is of recent, lightweight and ductile construction which has typically performed well in the Canterbury earthquake sequence.

## 6 Conclusions and Recommendations

Given the good performance of the Canterbury Agricultural Park Toilets in the Canterbury earthquake sequence, the lack of foundation damage and the floor levels considered to be within acceptable limits, **a geotechnical investigation is currently not considered necessary.**

Additionally, the building has suffered no loss of functionality and in our opinion the Canterbury Agricultural Park Toilets **is considered suitable for continued occupation.**

## 7 Explanatory Statement

The inspections of the building discussed in this report have been undertaken to assess structural earthquake damage. No analysis has been undertaken to assess the strength of the building or to determine whether or not it complies with the relevant building codes, except to the extent that Aurecon expressly indicates otherwise in the report. Aurecon has not made any assessment of structural stability or building safety in connection with future aftershocks or earthquakes – which have the potential to damage the building and to jeopardise the safety of those either inside or adjacent to the building, except to the extent that Aurecon expressly indicates otherwise in the report.

This report is necessarily limited by the restricted ability to carry out inspections due to potential structural instabilities/safety considerations, and the time available to carry out such inspections. The report does not address defects that are not reasonably discoverable on visual inspection, including defects in inaccessible places and latent defects. Where site inspections were made, they were restricted to external inspections and, where practicable, limited internal visual inspections.

While this report may assist the client in assessing whether the building should be repaired, strengthened, or replaced that decision is the sole responsibility of the client.

This review has been prepared by Aurecon at the request of its client and is exclusively for the client's use. It is not possible to make a proper assessment of this review without a clear understanding of the terms of engagement under which it has been prepared, including the scope of the instructions and directions given to and the assumptions made by Aurecon. The report will not address issues which would need to be considered for another party if that party's particular circumstances, requirements and experience were known and, further, may make assumptions about matters of which a third party is not aware. No responsibility or liability to any third party is accepted for any loss or damage whatsoever arising out of the use of or reliance on this report by any third party.

Without limiting any of the above, Aurecon's liability, whether under the law of contract, tort, statute, equity or otherwise, is limited as set out in the terms of the engagement with the client.

# Appendices



# Appendix A

## Site Location, Photos and Levels Survey

21 August 2012 – Canterbury Agricultural Park Toilets Site Photographs








Aerial Photograph courtesy of the Canterbury Geotechnical Database

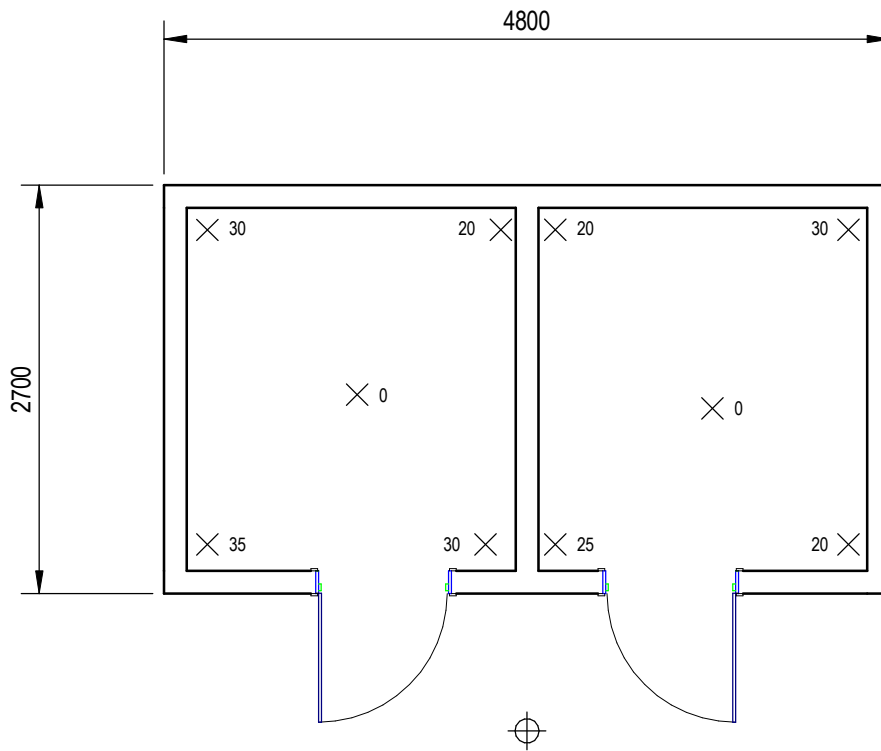
Eastern (front oblique) elevation of the Canterbury Agricultural Park Toilets.



Western (end oblique) elevation of the Canterbury Agricultural Park Toilets.



<p>Front view of the Canterbury Agricultural Park Toilets.</p>	
<p>Canterbury Agricultural Park Toilets foundations.</p>	
<p>Interior view of the Canterbury Agricultural Park Toilets.</p>	
<p>Interior view of the Canterbury Agricultural Park Toilets.</p>	
<p>Wall thickness approximately 120mm thick.</p>	



29/08/2017 4:52:17 pm



REV	DATE	REVISION DETAILS	APPROVAL

DRAWN	DESIGNED
D.HUNIA	C.BONG
CHECKED	
L.CASTILLO	
APPROVED	
	DATE
L.CASTILLO	

PROJECT
CANTERBURY AGRICULTURAL PARK TOILETS
TITLE
LEVEL SURVEY

PRELIMINARY NOT FOR CONSTRUCTION	
PROJECT No. 231550	
SCALE 1:50	SIZE A4
DRAWING No. S-01-02	REV

# Appendix B

## References

1. Department of Building and Housing (DBH), “Revised Guidance on Repairing and Rebuilding Houses Affected by the Canterbury Earthquake Sequence”, November 2011
2. New Zealand Society for Earthquake Engineering (NZSEE), “Assessment and Improvement of the Structural Performance of Buildings in Earthquakes”, April 2012
3. Standards New Zealand, “AS/NZS 1170 Part 0, Structural Design Actions: General Principles”, 2002
4. Standards New Zealand, “AS/NZS 1170 Part 1, Structural Design Actions: Permanent, imposed and other actions”, 2002
5. Standards New Zealand, “NZS 1170 Part 5, Structural Design Actions: Earthquake Actions – New Zealand”, 2004
6. Standards New Zealand, “NZS 3101 Part 1, The Design of Concrete Structures”, 2006
7. Standards New Zealand, “NZS 3404 Part 1, Steel Structures Standard”, 1997
8. Standards New Zealand, “NZS 3603, Timber Structures Standard”, 1993
9. Standards New Zealand, “NZS 3604, Timber Framed Structures”, 2011
10. Standards New Zealand, “NZS 4229, Concrete Masonry Buildings Not Requiring Specific Engineering Design”, 1999
11. Standards New Zealand, “NZS 4230, Design of Reinforced Concrete Masonry Structures”, 2004

# Appendix C

## Strength Assessment Explanation

### New building standard (NBS)

New building standard (NBS) is the term used with reference to the earthquake standard that would apply to a new building of similar type and use if the building was designed to meet the latest design Codes of Practice. If the strength of a building is less than this level, then its strength is expressed as a percentage of NBS.

### Earthquake Prone Buildings

A building can be considered to be earthquake prone if its strength is less than one third of the strength to which an equivalent new building would be designed, that is, less than 33%NBS (as defined by the New Zealand Building Act). If the building strength exceeds 33%NBS but is less than 67%NBS the building is considered at risk.

### Christchurch City Council Earthquake Prone Building Policy 2010

The Christchurch City Council (CCC) already had in place an Earthquake Prone Building Policy (EPB Policy) requiring all earthquake-prone buildings to be strengthened within a timeframe varying from 15 to 30 years. The level to which the buildings were required to be strengthened was 33%NBS.

As a result of the 4 September 2010 Canterbury earthquake the CCC raised the level that a building was required to be strengthened to from 33% to 67% NBS but qualified this as a target level and noted that the actual strengthening level for each building will be determined in conjunction with the owners on a building-by-building basis. Factors that will be taken into account by the Council in determining the strengthening level include the cost of strengthening, the use to which the building is put, the level of danger posed by the building, and the extent of damage and repair involved.

Irrespective of strengthening level, the threshold level that triggers a requirement to strengthen is 33%NBS.

As part of any building consent application fire and disabled access provisions will need to be assessed.

### Christchurch Seismicity

The level of seismicity within the current New Zealand loading code (AS/NZS 1170) is related to the seismic zone factor. The zone factor varies depending on the location of the building within NZ. Prior to the 22<sup>nd</sup> February 2011 earthquake the zone factor for Christchurch was 0.22. Following the earthquake the seismic zone factor (level of seismicity) in the Christchurch and surrounding areas has been increased to 0.3. This is a 36% increase.

For this assessment, the building's earthquake resistance is compared with the current New Zealand Building Code requirements for a new building constructed on the site. This is expressed as a percentage of new building standard (%NBS). The new building standard load requirements have been determined in accordance with the current earthquake loading standard (NZS 1170.5:2004 Structural design actions - Earthquake actions - New Zealand).

The likely capacity of this building has been derived in accordance with the New Zealand Society for Earthquake Engineering (NZSEE) guidelines 'Assessment and Improvement of the Structural Performance of Buildings in Earthquakes' (AISPBE), 2006. These guidelines provide an Initial Evaluation Procedure that

assesses a buildings capacity based on a comparison of loading codes from when the building was designed and currently. It is a quick high-level procedure that can be used when undertaking a Qualitative analysis of a building. The guidelines also provide guidance on calculating a modified Ultimate Limit State capacity of the building which is much more accurate and can be used when undertaking a Quantitative analysis.

The New Zealand Society for Earthquake Engineering has proposed a way for classifying earthquake risk for existing buildings in terms of %NBS and this is shown in Figure C1 below.

Description	Grade	Risk	%NBS	Existing Building Structural Performance	Improvement of Structural Performance	
					Legal Requirement	NZSEE Recommendation
Low Risk Building	A or B	Low	Above 67	Acceptable (improvement may be desirable)	The Building Act sets no required level of structural improvement (unless change in use) This is for each TA to decide. Improvement is not limited to 34%NBS.	100%NBS desirable. Improvement should achieve at least 67%NBS
Moderate Risk Building	B or C	Moderate	34 to 66	Acceptable legally. Improvement recommended		Not recommended. Acceptable only in exceptional circumstances
High Risk Building	D or E	High	33 or lower	Unacceptable (Improvement	Unacceptable	Unacceptable

Figure C1: NZSEE Risk Classifications Extracted from table 2.2 of the NZSEE 2006 AISPBE Guidelines

Table C1 below compares the percentage NBS to the relative risk of the building failing in a seismic event with a 10% probability of exceedance in 50 years (i.e. 0.2% in the next year). It is noted that the current seismic risk in Christchurch results in a 6% probability of exceedance in the next year.

Table C1: Relative Risk of Building Failure In A

Percentage of New Building Standard (%NBS)	Relative Risk (Approximate)
>100	<1 time
80-100	1-2 times
67-80	2-5 times
33-67	5-10 times
20-33	10-25 times
<20	>25 times



# Appendix D

## Background and Legal Framework

### Background

Aurecon has been engaged by the Christchurch City Council (CCC) to undertake a detailed engineering evaluation of the building

This report is a Qualitative Assessment of the building structure, and is based on the Detailed Engineering Evaluation Procedure document (draft) issued by the Structural Advisory Group on 19 July 2011.

A qualitative assessment involves inspections of the building and a desktop review of existing structural and geotechnical information, including existing drawings and calculations, if available.

The purpose of the assessment is to determine the likely building performance and damage patterns, to identify any potential critical structural weaknesses or collapse hazards, and to make an initial assessment of the likely building strength in terms of percentage of new building standard (%NBS).

### Compliance

This section contains a brief summary of the requirements of the various statutes and authorities that control activities in relation to buildings in Christchurch at present.

### Canterbury Earthquake Recovery Authority (CERA)

CERA was established on 28 March 2011 to take control of the recovery of Christchurch using powers established by the Canterbury Earthquake Recovery Act enacted on 18 April 2011. This act gives the Chief Executive Officer of CERA wide powers in relation to building safety, demolition and repair. Two relevant sections are:

#### **Section 38 – Works**

This section outlines a process in which the chief executive can give notice that a building is to be demolished and if the owner does not carry out the demolition, the chief executive can commission the demolition and recover the costs from the owner or by placing a charge on the owners' land.

#### **Section 51 – Requiring Structural Survey**

This section enables the chief executive to require a building owner, insurer or mortgagee carry out a full structural survey before the building is re-occupied.

We understand that CERA will require a detailed engineering evaluation to be carried out for all buildings (other than those exempt from the Earthquake Prone Building definition in the Building Act). It is anticipated that CERA will adopt the Detailed Engineering Evaluation Procedure document (draft) issued by the Structural Advisory Group on 19 July 2011. This document sets out a methodology for both qualitative and quantitative assessments.

The qualitative assessment is a desk-top and site inspection assessment. It is based on a thorough visual inspection of the building coupled with a review of available documentation such as drawings and specifications. The quantitative assessment involves analytical calculation of the buildings strength and may require non-destructive or destructive material testing, geotechnical testing and intrusive investigation.

It is anticipated that factors determining the extent of evaluation and strengthening level required will include:

- The importance level and occupancy of the building
- The placard status and amount of damage
- The age and structural type of the building
- Consideration of any critical structural weaknesses
- The extent of any earthquake damage

## Building Act

Several sections of the Building Act are relevant when considering structural requirements:

### Section 112 – Alterations

This section requires that an existing building complies with the relevant sections of the Building Code to at least the extent that it did prior to any alteration. This effectively means that a building cannot be weakened as a result of an alteration (including partial demolition).

### Section 115 – Change of Use

This section requires that the territorial authority (in this case Christchurch City Council (CCC)) be satisfied that the building with a new use complies with the relevant sections of the Building Code 'as near as is reasonably practicable'. Regarding seismic capacity 'as near as reasonably practicable' has previously been interpreted by CCC as achieving a minimum of 67%NBS however where practical achieving 100%NBS is desirable. The New Zealand Society for Earthquake Engineering (NZSEE) recommend a minimum of 67%NBS.

### Section 121 – Dangerous Buildings

The definition of dangerous building in the Act was extended by the Canterbury Earthquake (Building Act) Order 2010, and it now defines a building as dangerous if:

- in the ordinary course of events (excluding the occurrence of an earthquake), the building is likely to cause injury or death or damage to other property; or
- in the event of fire, injury or death to any persons in the building or on other property is likely because of fire hazard or the occupancy of the building; or
- there is a risk that the building could collapse or otherwise cause injury or death as a result of earthquake shaking that is less than a 'moderate earthquake' (refer to Section 122 below); or
- there is a risk that that other property could collapse or otherwise cause injury or death; or
- a territorial authority has not been able to undertake an inspection to determine whether the building is dangerous.

### Section 122 – Earthquake Prone Buildings

This section defines a building as earthquake prone if its ultimate capacity would be exceeded in a 'moderate earthquake' and it would be likely to collapse causing injury or death, or damage to other property. A moderate earthquake is defined by the building regulations as one that would generate ground shaking 33% of the shaking used to design an equivalent new building.

## Section 124 – Powers of Territorial Authorities

This section gives the territorial authority the power to require strengthening work within specified timeframes or to close and prevent occupancy to any building defined as dangerous or earthquake prone.

## Section 131 – Earthquake Prone Building Policy

This section requires the territorial authority to adopt a specific policy for earthquake prone, dangerous and insanitary buildings.

## Christchurch City Council Policy

Christchurch City Council adopted their Earthquake Prone, Dangerous and Insanitary Building Policy in 2006. This policy was amended immediately following the Darfield Earthquake of the 4th September 2010.

The 2010 amendment includes the following:

- A process for identifying, categorising and prioritising Earthquake Prone Buildings, commencing on 1 July 2012;
- A strengthening target level of 67% of a new building for buildings that are Earthquake Prone;
- A timeframe of 15-30 years for Earthquake Prone Buildings to be strengthened; and,
- Repair works for buildings damaged by earthquakes will be required to comply with the above.

The council has stated their willingness to consider retrofit proposals on a case by case basis, considering the economic impact of such a retrofit.

We anticipate that any building with a capacity of less than 33%NBS (including consideration of critical structural weaknesses) will need to be strengthened to a target of 67%NBS of new building standard as recommended by the Policy.

If strengthening works are undertaken, a building consent will be required. A requirement of the consent will require upgrade of the building to comply 'as near as is reasonably practicable' with:

- The accessibility requirements of the Building Code.
- The fire requirements of the Building Code. This is likely to require a fire report to be submitted with the building consent application.

## Building Code

The building code outlines performance standards for buildings and the Building Act requires that all new buildings comply with this code. Compliance Documents published by The Department of Building and Housing can be used to demonstrate compliance with the Building Code.

After the February Earthquake, on 19 May 2011, Compliance Document B1: Structure was amended to include increased seismic design requirements for Canterbury as follows:

- Hazard Factor increased from 0.22 to 0.3 (36% increase in the basic seismic design load)
- Serviceability Return Period Factor increased from 0.25 to 0.33 (80% increase in the serviceability design loads when combined with the Hazard Factor increase)

The increase in the above factors has resulted in a reduction in the level of compliance of an existing building relative to a new building despite the capacity of the existing building not changing.

# Appendix E

## Standard Reporting Spread Sheet

Detailed Engineering Evaluation Summary Data

V1.11

<b>Location</b>		Building Name: <input type="text" value="Canterbury Agricultural Park Toilets"/>	Reviewer: <input type="text" value="Lee Howard"/>
	Unit No: Street		CPEng No: <input type="text" value="1008889"/>
Building Address: <input type="text" value="61 Wigram Road"/>		Company: <input type="text" value="Aurecon NZ Ltd"/>	
Legal Description: <input type="text" value="Lot 1 DP 302504"/>		Company project number: <input type="text" value="231550"/>	
		Company phone number: <input type="text" value="03 366 0821"/>	
	Degrees Min Sec	Date of submission: <input type="text" value="R   -1H"/>	
GPS south: <input type="text" value="43 33 6.80"/>		Inspection Date: <input type="text" value="Aug-12"/>	
GPS east: <input type="text" value="172 35 0.50"/>		Revision: <input type="text" value="GA"/>	
Building Unique Identifier (CCC): <input type="text" value="PRK_2412 BLDG 002"/>		Is there a full report with this summary? <input type="text" value="yes"/>	

<b>Site</b>		Site slope: <input type="text" value="flat"/>	Max retaining height (m): <input type="text" value="0"/>
	Soil type: <input type="text" value="mixed"/>	Soil Profile (if available): <input type="text"/>	
Site Class (to NZS1170.5): <input type="text" value="D"/>		If Ground improvement on site, describe: <input type="text"/>	
Proximity to waterway (m, if <100m): <input type="text"/>		Approx site elevation (m): <input type="text" value="17.00"/>	
Proximity to cliff top (m, if < 100m): <input type="text"/>			
Proximity to cliff base (m, if <100m): <input type="text"/>			

<b>Building</b>		No. of storeys above ground: <input type="text" value="1"/>	single storey = 1	Ground floor elevation (Absolute) (m): <input type="text" value="17.20"/>
Ground floor split? <input type="text" value="no"/>		Stores below ground: <input type="text" value="0"/>		Ground floor elevation above ground (m): <input type="text" value="0.20"/>
Foundation type: <input type="text" value="raft slab"/>		Building height (m): <input type="text" value="3.30"/>	if Foundation type is other, describe: <input type="text"/>	
Floor footprint area (approx): <input type="text" value="13"/>		Age of Building (years): <input type="text" value="13"/>	height from ground to level of uppermost seismic mass (for IEP only) (m): <input type="text" value="3.3"/>	
Strengthening present? <input type="text" value="no"/>			Date of design: <input type="text" value="2004-"/>	
Use (ground floor): <input type="text" value="public"/>			If so, when (year)? <input type="text"/>	
Use (upper floors): <input type="text"/>			And what load level (%g)? <input type="text"/>	
Use notes (if required): <input type="text" value="toilets"/>			Brief strengthening description: <input type="text"/>	
Importance level (to NZS1170.5): <input type="text" value="IL1"/>				

<b>Gravity Structure</b>		Gravity System: <input type="text" value="load bearing walls"/>	rafter type, purlin type and cladding: <input type="text" value="timber purlins and rafters, profiled metal sheet roof"/>
Roof: <input type="text" value="timber framed"/>		Floors: <input type="text" value="concrete flat slab"/>	slab thickness (mm): <input type="text" value="unknown"/>
Beams: <input type="text" value="timber"/>		Columns: <input type="text" value="load bearing walls"/>	typical dimensions (mm x mm): <input type="text" value="timber lintels"/>
Walls: <input type="text"/>			

<b>Lateral load resisting structure</b>		Lateral system along: <input type="text" value="lightweight timber framed walls"/>	<b>Note: Define along and across in detailed report!</b>	<input type="text" value="3"/>
Ductility assumed, $\mu$ :	<input type="text" value="3.00"/>	Period along: <input type="text" value="0.40"/>	note typical wall length (m)	
Total deflection (ULS) (mm):		maximum interstorey deflection (ULS) (mm): <input type="text"/>	estimate or calculation? <input type="text" value="estimated"/>	
			estimate or calculation? <input type="text"/>	
			estimate or calculation? <input type="text"/>	

Lateral system across:

Ductility assumed,  $\mu$ :

Period across:

Total deflection (ULS) (mm):

maximum interstorey deflection (ULS) (mm):

0.00

note typical wall length (m)

estimate or calculation?

estimate or calculation?

estimate or calculation?

**Separations:**

north (mm):

east (mm):

south (mm):

west (mm):

leave blank if not relevant

**Non-structural elements**

Stairs:

Wall cladding:

Roof Cladding:

Glazing:

Ceilings:

Services(list):

describe

**Available documentation**

Architectural:

Structural:

Mechanical:

Electrical:

Geotech report:

original designer name/date

original designer name/date

original designer name/date

original designer name/date

original designer name/date

**Damage**

Site:  
(refer DEE Table 4-2)

Site performance:

Describe damage:

Settlement:

Differential settlement:

Liquefaction:

Lateral Spread:

Differential lateral spread:

Ground cracks:

Damage to area:

notes (if applicable):

notes (if applicable):

notes (if applicable):

notes (if applicable):

notes (if applicable):

notes (if applicable):

**Building:**

Current Placard Status:

Along

Damage ratio:

Describe (summary):

Describe how damage ratio arrived at:

Across

Damage ratio:

Describe (summary):

$$Damage\_Ratio = \frac{(\%NBS(before) - \%NBS(after))}{\%NBS(before)}$$

Diaphragms

Damage?:

Describe:

CSWs:

Damage?:

Describe:

Pounding:

Damage?:

Describe:

Non-structural:

Damage?:

Describe:

**Recommendations**

Level of repair/strengthening required:   
 Building Consent required:   
 Interim occupancy recommendations:

Describe:   
 Describe:   
 Describe:

Along Assessed %NBS before e'quakes:  ##### %NBS from IEP below  
 Assessed %NBS after e'quakes:

If IEP not used, please detail assessment methodology:

Across Assessed %NBS before e'quakes:  ##### %NBS from IEP below  
 Assessed %NBS after e'quakes:

**IEP**

**Use of this method is not mandatory - more detailed analysis may give a different answer, which would take precedence. Do not fill in fields if not using IEP.**

Period of design of building (from above): 2004-

$h_n$  from above: 3.3m

Seismic Zone, if designed between 1965 and 1992:

Design Soil type from NZS1170.5:2004, cl 3.1.3:  
 not required for this age of building

	along	across
Period (from above):	0.4	0.4
(%NBS) <sub>nom</sub> from Fig 3.3:	<input type="text"/>	<input type="text"/>

Note:1 for specifically design public buildings, to the code of the day: pre-1965 = 1.25; 1965-1976, Zone A =1.33; 1965-1976, Zone B = 1.2; all else 1.0

Note 2: for RC buildings designed between 1976-1984, use 1.2

Note 3: for buildngs designed prior to 1935 use 0.8, except in Wellington (1.0)

	along	across
<b>Final (%NBS)<sub>nom</sub>:</b>	<b>0%</b>	<b>0%</b>

**2.2 Near Fault Scaling Factor**

Near Fault scaling factor, from NZS1170.5, cl 3.1.6:

	along	across
Near Fault scaling factor (1/N(T,D), <b>Factor A:</b>	#DIV/0!	#DIV/0!

**2.3 Hazard Scaling Factor**

Hazard factor Z for site from AS1170.5, Table 3.3:

$Z_{1992}$ , from NZS4203:1992   
 Hazard scaling factor, **Factor B:**

**2.4 Return Period Scaling Factor**

Building Importance level (from above):   
 Return Period Scaling factor from Table 3.1, **Factor C:**

**2.5 Ductility Scaling Factor**

	along	across
Assessed ductility (less than max in Table 3.2) <input type="text"/>		
Ductility scaling factor: =1 from 1976 onwards; or = $k_\mu$ , if pre-1976, from Table 3.3: <input type="text"/>		

Ductility Scaling Factor, <b>Factor D:</b>	1.00	1.00
--	------	------

**2.6 Structural Performance Scaling Factor:**

Sp:

Structural Performance Scaling Factor <b>Factor E:</b>	#DIV/0!	#DIV/0!
--	---------	---------

**2.7 Baseline %NBS,  $(NBS\%)_b = (NBS)_{nom} \times A \times B \times C \times D \times E$**

<b>%NBS<sub>b</sub>:</b>	<b>#DIV/0!</b>	<b>#DIV/0!</b>
--------------------------	----------------	----------------

Global Critical Structural Weaknesses: (refer to NZSEE IEP Table 3.4)

3.1. Plan Irregularity, factor A:

3.2. Vertical irregularity, Factor B:

3.3. Short columns, Factor C:

3.4. Pounding potential  
Pounding effect D1, from Table to right   
Height Difference effect D2, from Table to right

Therefore, Factor D:

3.5. Site Characteristics

Table for selection of D1	Severe	Significant	Insignificant/none
	Separation 0<sep<.005H	.005<sep<.01H	Sep>.01H
Alignment of floors within 20% of H	<b>0.7</b>	<b>0.8</b>	<b>1</b>
Alignment of floors not within 20% of H	<b>0.4</b>	<b>0.7</b>	<b>0.8</b>

Table for Selection of D2	Severe	Significant	Insignificant/none
	Separation 0<sep<.005H	.005<sep<.01H	Sep>.01H
Height difference > 4 storeys	0.4	0.7	1
Height difference 2 to 4 storeys	0.7	0.9	1
Height difference < 2 storeys	1	1	1

3.6. Other factors, Factor F For  $\leq 3$  storeys, max value =2.5, otherwise max valule =1.5, no minimum   
Rationale for choice of F factor, if not 1

Detail Critical Structural Weaknesses: (refer to DEE Procedure section 6)  
List any:  Refer also section 6.3.1 of DEE for discussion of F factor modification for other critical structural weaknesses

3.7. Overall Performance Achievement ratio (PAR)

4.3 PAR x (%NBS)b: PAR x Baseline %NBS:

4.4 Percentage New Building Standard (%NBS), (before)





**Aurecon New Zealand Limited**  
**Level 2, 518 Colombo Street**  
**Christchurch 8011**

PO Box 1061  
Christchurch 8140  
New Zealand

**T** +64 3 366 0821

**F** +64 3 379 6955

**E** [christchurch@aurecongroup.com](mailto:christchurch@aurecongroup.com)

**W** [aurecongroup.com](http://aurecongroup.com)

Aurecon offices are located in:  
Angola, Australia, Botswana, China,  
Ethiopia, Hong Kong, Indonesia,  
Lesotho, Libya, Malawi, Mozambique,  
Namibia, New Zealand, Nigeria,  
Philippines, Singapore, South Africa,  
Swaziland, Tanzania, Thailand, Uganda,  
United Arab Emirates, Vietnam.